

## APPENDIX A: SCOPE OF WORK

## **Stadium Shopping Centre Area Redevelopment Plan**

Transportation Impact Assessment | FINAL Scope of Work | 2013-03-06

### **1) PREAMBLE**

- A. Western Securities is proposing to redevelop the Stadium Shopping Centre into a mixed use site with office, retail and residential land uses. It is our understanding that the City is leading an Area Redevelopment Plan (ARP) for the site and will require transportation analysis to support the plan.
- B. DA Watt has been retained by Western Securities to provide this information in support of the ARP and proposes the following scope of work for the Transportation Impact Assessment (TIA).

### **2) HORIZON YEARS**

- A. Existing Conditions (Baseline)
- B. Future Conditions (2039)
- C. Counts shall include vehicular, bicycle, and pedestrian traffic. DA Watt shall provide balanced traffic volumes between intersections that are going to be analysed for both existing and future time horizons.

### **3) TIME PERIODS**

- A. Weekday AM/PM Peak Hours reflective of the timing of Foothills Medical Centre shift changes and Westmount Charter School start/end times.
- B. DA Watt shall show 24-hour data for the roadways in the area to confirm that peak periods are being picked up.
- C. DA Watt shall count Uxbridge and the City will update 16 Avenue volumes if necessary.

### **4) INTERSECTION CAPACITY ANALYSIS**

- A. 16 Avenue & Uxbridge Drive
- B. Site Access (South)
- C. Site Access (North) – existing horizon only
- D. Site Access (North – Unwin Road (Lane) / Uxbridge Drive)
- E. Uxbridge Drive and Lane (on the west side of the Queen of Peace Church)
- F. Unwin Road & University Drive
- G. Volume shifts from Tim Horton's/Shell resulting from project and potential modifications to Uxbridge Drive shall be included in analysis of study area intersections.
- H. In addition to Synchro analysis, SIDRA roundabout analysis shall be done for intersections of Uxbridge Dr & Unwin Rd NW / lane north of site and Uxbridge Dr & Ulster Rd NW in comparing and recommending ultimate intersection control and configuration at these locations.

### **5) WEAVING ANALYSIS**

- A. DA Watt shall review existing and future conditions of weaving area westbound on 16 Avenue between University Drive and Uxbridge Drive during AM and PM peak hours.

### **6) SENSITIVITY ANALYSIS**

- A. DA Watt shall conduct sensitivity analysis for all of the study intersections based on a more conservative mode split data (assuming more people driving). The recommended numbers to be used will be verified with the City prior to the analysis.

**7) REVIEW OF MODEL DATA**

- A. DA Watt shall review City's Regional Transportation Model (RTM) data to check population and growth forecasts for Foothills Medical Centre, West Campus, the University of Calgary, McMahon Stadium and the Foothills Athletic Park.
- B. As per the City's TIA guidelines, DA Watt shall include information about new developments in the area surrounding the proposed site (West campus, Foothills Medical Centre future expansion, Foothills Athletic centre expansion, McMahon redevelopment etc.) and projected traffic volumes in the background traffic. DA Watt shall provide comments on the relocation of Westmount Charter School (citywide catchment) to University Heights and the impacts of University Elementary School having a 30% non community enrolment.

**8) DAILY VOLUME ANALYSIS**

- A. Uxbridge Drive
- B. Unwin Road and the lane to the north of the site
- C. 16 Avenue
- D. Significant internal roads

**9) MODE SPLIT DATA**

- A. Current mode split data for site (based on survey to be conducted by DA Watt)
- B. Discussion on mode split for future (full-build scenario)
- C. DA Watt to obtain RTM mode split and discuss implications

**10) PROPOSED TRIP GENERATION RATES**

- A. 16 Avenue west of Uxbridge Drive is part of the future primary transit network, therefore DA Watt shall use the following mix of TOD and standard rates for the proposed land uses as follows:
- B. TOD Office: 1.39 (AM) / 1.57 (PM) – City of Calgary Standard Rate
- C. Medical Office: 3.00 (AM) / 4.0 (PM) – Local Example (Ashton Professional Centre)
- D. Retail: 2.00 (AM) / 6.00 (PM) – City of Calgary
- E. Hotel: 0.56 (AM) / 0.59 (PM) - ITE
- F. TOD Residential: 0.35 (AM) / 0.45 (PM) – City of Calgary Standard Rate

**11) TRIP ASSIGNMENT AND DISTRIBUTION**

- A. DA Watt shall estimate the vehicular, bicycle, and pedestrian trip generation for the proposed site for the AM and PM peak periods, and assign all trips to the adjacent roadways.
- B. The City's RTM select zone analysis shall be used for the trip distribution patterns. If some minor adjustments need to be done this has to be approved by the City.

**12) SITE ACCESS REVIEW**

- A. DA Watt shall assess the operation of the site access locations, and provide commentary related to on-site circulation.
- B. South site access proximity to the signalized intersection of 16 Avenue & Uxbridge Drive NW shall be included in this review.

**13) INTERNAL TRIP CAPTURE, PASS-BY, AND DIVERTED LINKED TRIPS**

- A. DA Watt shall use Transportation Research Board methodology for calculating internal trip rates.

- B. The City estimates that 20% of the external trips to/from the site will be either diverted linked trips (drivers already travelling along 16 Avenue that divert to the site and then continue on 16 Avenue) or pass-by trips (drivers already travelling along Uxbridge Drive that turn into the site and then continue along Uxbridge Drive). DA Watt shall document the split between pass-by and diverted linked trips in the TIA report.

#### **14) PARKING REVIEW**

- A. DA Watt shall review on- and off-site parking requirements and management issues, including: bylaw requirements and justification for any reduction, hospital traffic currently using the parking lot and where they will be relocated to, and recommendations for neighbourhood spillover parking mitigation.

#### **15) TRANSIT**

- A. Existing Transit Service & Conditions
- B. Current Pedestrian Facilities
- C. Current Pedestrian Volumes
- D. Future Transit Service Planned for the Immediate Area (assessment of what's needed to meet the anticipated demand)
- E. Forecast of transit trips generated to/from the site using the forecast mode split
- F. Proposed transit facilities for site and surrounding area (bus shelters, benches, bus bays connections etc.)

#### **16) ACTIVE MODES**

- A. DA Watt shall review the HCM 2010 methodology for assessing bikes/peds and will provide the City with an assessment of whether this analysis will add value to the process and at what cost.
- B. DA Watt shall provide an assessment of available infrastructure in the area surrounding the site for pedestrians and cyclists (with photos)
- C. DA Watt shall map out destinations, routes, and barriers within 1km and 3km of the site, including the identification of potential improvements
- D. DA Watt shall estimate future peak hour pedestrian volumes generated by the site

#### **17) SHORT-CUTTING TRAFFIC REVIEW**

- A. DA Watt shall review the potential impacts of the site on short-cutting traffic by:
  - a. Identifying potential routes
  - b. Estimating the potential increase in volumes
  - c. Develop potential mitigation measures including closures, turn restrictions, and both horizontal and vertical calming installations

#### **18) TDM**

- A. DA Watt shall comment on the high level characteristics of a TDM plan that is appropriate for the site and things that can be included in conditions of development (i.e. carpool parking, bike lockers, shower facilities, car share program etc.).
- B. Include recommendations for design/improvements as required for all report sections (e.g. access, parking, proposed road cross-sections, and TDM measures)

#### **19) PROJECT MANAGEMENT**

- A. The City and DA Watt shall meet at least every two weeks to discuss the progress and results. Some additional work might be required after these consultations.

**Project: STADIUM SHOPPING CENTRE –TIA: Version 3**  
**RE: PROPOSED SCOPE OF WORK**

Western Securities is proposing to redevelop the Stadium Shopping Centre into a mixed use site with office, retail and residential land uses. It is our understanding that the City is leading an Area Redevelopment Plan Amendment (ARPA) for the site and will require transportation analysis to support the plan. DAW has been retained by Western Securities to provide this information in support of the ARPA. We propose the following scope of work for the Transportation Impact Assessment (TIA).

**1) HORIZON YEARS**

- Existing Conditions (Baseline)
- Future - 2039

Counts shall include the vehicular, bicycle, and pedestrian traffic. Please provide balanced traffic volumes between intersections that are going to be analysed for both existing and future time horizons.

**2) TIME PERIODS**

- Weekday AM/PM Peak Hours
- We will show 24-hour data for the roadways in the area to confirm that we are picking up the peak periods (we will count Uxbridge and the City will update 16th Avenue volumes if required)

**3) INTERSECTION CAPACITY ANALYSIS**

- 16th Avenue & Uxbridge Drive
- Site Access (South)
- Site Access (North) – existing horizon only
- Site Access (North –Unwin Road(Lane)/ Uxbridge Drive)
- Uxbridge Drive and Lane (on west side of Queen of Peace Church)
- Unwin Road & University Drive
- Volume shifts from Tim Horton's/Gas Station resulting from project to be included in analysis of study area intersections.

In addition to Synchro, SIDRA roundabout analysis is to be done for intersections of Uxbridge Dr & Unwin Rd NW/ lane north of site and Uxbridge Dr & Ulster Rd NW in comparing and recommending ultimate intersection control and configuration at these locations.

**4) WEAVING ANALYSIS**

- Review existing and future conditions of weaving area westbound on 16th Avenue between University Drive and Uxbridge Drive during a.m. and p.m. peak hours

**5) SENSITIVITY ANALYSIS**

- Conduct sensitivity analysis for all of the study intersections based on a more conservative mode split data (more people driving). The recommended number used will be verified with the City prior to the analysis.

**6) REVIEW OF MODEL DATA**

- Review City's Regional Transportation Model (RTM) data to see what has been assumed for Foothills Medical Centre, West Campus, Uo C, McMahon Stadium and the Foothills Athletic Park.
- Add a discussion in the report that addresses the above information.

As per TIA guidelines, include information about new developments in the area surrounding the proposed site (like West campus, Foothills Hospital future expansion, Foothills Athletic centre expansion, McMahon redevelopment etc.) and projected traffic volumes in the background traffic. Provide comments on the relocation of Westmount Charter School to University Heights, including University Elementary School having a 30% non community enrolment.

## **7) DAILY VOLUME ANALYSIS**

- Uxbridge Drive
- Unwin Road
- 16th Avenue
- Significant Internal roads
- Lane north of site

## **8) MODE SPLIT DATA**

- Current mode split data for site (based on survey)
- Discussion on mode split for future (full-build scenario)
- Obtain RTM Mode Split and provide discussion

## **9) PROPOSED TRIP GENERATION RATES**

16<sup>th</sup> Avenue west of Uxbridge Drive is part of the future primary transit network. Therefore we propose to use the following mix of TOD and standard rates for the proposed land uses as follows:

- TOD Office: 1.39 (AM) / 1.57 (PM) – City of Calgary Standard Rate
- Medical Office: 3.00 (AM) / 4.0 (PM) – Calgary Example (Ashton Professional Centre)
- Retail: **2.00** (AM) / 6.00 (PM) – City of Calgary
- Hotel: 0.56 (AM) / 0.59 (PM) - ITE
- TOD Residential: 0.35 (AM) / 0.45 (PM) – City of Calgary Standard Rate

Estimate the vehicular, bicycle, and pedestrian trip generation for the proposed site for the a.m. and p.m. peak periods, and assign all trips to the adjacent roadways

## **Needs a section for TRIP DISTRIBUTION**

City's Regional Transportation Model (RTM) select zone analysis shall be used for the trip distribution patterns. If some minor adjustments need to be done this has to be approved by TDS.

## **Needs a section for Site Access Review**

Assessment of the operation of the site access locations, and a commentary related to on-site circulation. South site access proximity to the signalized intersection of 16th Avenue & Uxbridge Drive NW shall be reviewed and commented on too.

## **10) INTERNAL TRIPS**

- Propose to use TRB method for calculating internal trip rates

## **11) PASS-BY AND DIVERTED LINKED TRIPS**

• We estimate that 20% of the external trips to/from the site will be either diverted linked trips (drivers already travelling along 16th Avenue that divert to the site and then continue on 16th Avenue) or pass-by trips (drivers already travelling along Uxbridge Drive that turn into the site and then continue along Uxbridge Drive). The split between pass-by and diverted linked trips will be documented in the TIA report.

## **Needs a section for Parking Review**

Please provide scope for this section – include bylaw requirement, justification for any reduction, discussion of hospital traffic currently using parking lot and where they will be relocated to; include recommendations for neighborhood parking mitigation if needed.

## **12) TRANSIT**

- Existing Transit Service & Conditions
- Current Pedestrian Facilities
- Current Pedestrian Volumes
- Future Transit Service Planned for the Immediate Area (assessment of what's needed to meet the anticipated demand)
- Forecast of transit trips generated to/from the site using the forecast mode split
- Proposed transit facilities for site and surrounding area (bus shelters, benches, bus bays connections etc.)

## **13) ACTIVE MODES**

- We will review the HCM 2010 methodology for assessing bikes/peds and will provide the City with an assessment of whether this analysis will add value to the process and at what cost.
- Description of Available Infrastructure in the area surrounding the site for pedestrians and cyclists (with photos)
- Figure of destinations within 1km and 3 km of site
- Discussion of Barriers and potential improvements
- Estimate of future peak hour pedestrian volumes generated by the site

## **14) SHORT-CUTTING TRAFFIC REVIEW**

- Review the potential impacts of the site on short-cutting traffic by
  - Identifying potential routes
  - Estimating the potential increase in volumes
  - Develop potential mitigation measures

## **15) TDM**

- Commentary on a TDM plan that is appropriate for the site and things that can be included in conditions of development (i.e. carpool parking, bike lockers, shower facilities, car share program etc.).

Include recommendations for design/improvements as required for all report sections (e.g. access, parking, proposed road cross-sections, and TDM measures)

**TDS and D.A. Watt will meet at least every two weeks to discuss the progress and results. Some additional work might be required after these consultations.**

## APPENDIX B: INTERSECTION COUNTS

Location : North Leg :

**East Leg:** Uxbridge Drive NW

West Leg: Uxbridge Drive NW

**South Leg :** Westmount School Access

Date : Wednesday February 13, 2013

Observer(s) : Bart

Job #: 2818.T02

Job Name: Stadium Shopping Centre TIA



Time Starting	From the North On:						From the South On: Westmount School Access						From the East On: Uxbridge Drive NW						From the West On: Uxbridge Drive NW						Total			Total Vehicles							
	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Cars	Trucks	Peds				
	Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks								
07:00	0	0	0	0	0	0	0	0	0	0	0	2	0	2	5	0	10	0	0	0	0	0	0	19	0	0	0	0	36	0	2	36			
07:15	0	0	0	0	0	0	0	0	0	0	0	1	0	1	3	0	8	0	0	0	0	0	0	0	17	0	0	0	0	29	0	1	29		
07:30	0	0	0	0	0	0	5	0	0	0	0	1	0	0	8	0	13	2	0	0	4	0	0	0	24	1	0	0	1	46	3	10	49		
07:45	0	0	0	0	0	0	0	2	0	0	0	5	0	6	20	0	22	0	0	0	1	0	0	0	33	1	1	0	0	83	1	7	84		
08:00	0	0	0	0	0	0	0	0	0	0	0	13	0	19	32	0	38	3	0	0	3	0	0	0	48	0	1	0	1	132	3	23	135		
08:15	0	0	0	0	0	0	0	0	0	0	0	17	0	21	49	0	62	3	0	0	5	0	0	0	68	11	3	0	0	199	14	26	213		
08:30	0	0	0	0	0	0	3	1	0	0	0	9	0	13	11	0	54	2	0	0	8	0	0	0	51	1	0	0	0	126	3	24	129		
08:45	0	0	0	0	0	0	0	0	0	0	0	0	0	1	8	0	41	3	0	0	7	0	0	0	69	5	0	0	0	118	8	8	126		
2 Hour Total	0	0	0	0	0	0	8	3	0	0	0	48	0	63	136	0	248	13	0	0	28	0	0	0	329	19	5	0	2	769	32	101	801		
Peak Hour: 8:00 - 9:00	0	0	0	0	0	0	3	1	0	0	0	39	0	54	100	0	195	11	0	0	23	0	0	0	236	17	4	0	1	575	28	81	603		
Total Veh & % Trucks	0	0%	0	0%	0	0%	1	0%	0	0%	39	0%	100	0%	206	5%	0	0%	0	0%	253	7%	4	0%	0	0	0	0	0	0	0	0	0		
PHF	0.00	Peak Total =						0	PHF	0.77	Peak Total =						40	PHF	1.05	Peak Total =						306	PHF	1.31	Peak Total =						257

Time Starting	From the North On:						From the South On: Westmount School Access						From the East On: Uxbridge Drive NW						From the West On: Uxbridge Drive NW						Total			Total Vehicles				
	Left Turn		Straight		Right Turn		Ped	Left Turn		Straight		Right Turn		Ped	Left Turn		Straight		Right Turn		Ped	Left Turn		Straight		Right Turn		Ped	Cars			Total Vehicles
	Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars			
03:00	0	0	0	0	0	0	0	0	0	0	0	4	2	0	3	4	36	0	0	0	9	0	0	26	0	0	0	0	69	6	9	75
03:15	0	0	0	0	0	0	1	0	0	0	14	1	24	5	3	53	2	0	0	13	0	0	34	2	1	0	0	108	8	37	116	
03:30	0	0	0	0	0	0	1	0	0	0	16	2	0	10	2	46	1	0	0	7	0	0	68	1	1	0	0	142	6	7	148	
03:45	0	0	0	0	0	0	1	0	0	0	14	1	0	1	2	38	1	0	0	3	0	0	35	1	1	0	0	90	5	3	95	
04:00	0	0	0	0	0	0	0	0	0	0	8	0	0	3	0	31	1	0	0	3	0	0	26	1	1	0	0	69	2	3	71	
04:15	0	0	0	0	0	0	0	0	0	0	9	0	0	1	0	25	0	0	0	2	0	0	15	0	1	0	0	51	0	2	51	
04:30	0	0	0	0	0	0	0	0	0	0	9	0	0	1	0	28	0	0	0	3	0	0	25	0	0	0	0	63	0	3	63	
04:45	0	0	0	0	0	0	0	0	0	0	7	0	0	2	0	25	0	0	0	7	0	0	33	0	1	0	0	68	0	7	68	
05:00	0	0	0	0	0	0	0	0	0	0	10	0	1	2	0	27	0	0	0	4	0	0	30	0	0	0	0	69	0	5	69	
05:15	0	0	0	0	0	0	0	0	0	0	5	0	1	0	0	24	0	0	0	3	0	0	22	0	0	0	0	51	0	4	51	
05:30	0	0	0	0	0	0	0	0	0	0	11	0	1	2	0	30	0	0	0	1	0	0	30	0	0	0	0	73	0	2	73	
05:45	0	0	0	0	0	0	0	0	0	0	5	0	0	2	0	29	0	0	0	3	0	0	21	0	0	0	0	57	0	3	57	
3 Hour Total	0	0	0	0	0	0	3	0	0	0	112	6	27	32	11	392	5	0	0	58	0	0	365	5	6	0	0	910	27	85	937	
Peak Hour: 3:00-4:00	0	0	0	0	0	0	3	0	0	0	48	6	24	19	11	173	4	0	0	32	0	0	163	4	3	0	0	409	25	56	434	
Total Veh & % Trucks	0	0%	0	0%	0	0%	3	0%	0	0%	54	11%	30	37%	177	2%	0	0%	0	0%	167	2%	3	0%	0							
	PHF	0.00					Peak Total =	0	PHF	1.43				Peak Total =	57	PHF	1.48				Peak Total =	207	PHF	1.25				Peak Total =	170			

Location : North Leg : Uxbridge Drive NW  
 South Leg : Uxbridge Drive NW  
 Date : Wednesday February 13, 2013  
 Observer(s) : Roland  
 Job # : 2818.T02  
 Job Name: Stadium Shopping Centre TIA



Time Starting	From the North On: Uxbridge Drive NW						From the South On: Uxbridge Drive NW						From the East On: Unwin Road NW						From the West On: Lane						Total			Total Vehicles				
	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Cars			Total Vehicles
	Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars						
07:00	2	0	17	0	0	0	1	1	0	7	0	11	2	1	42	2	2	0	9	0	2	0	0	0	0	0	5	91	4	9	95	
07:15	5	0	14	0	0	0	0	0	0	8	0	26	2	0	51	1	0	0	6	0	1	0	0	0	0	0	2	110	3	3	113	
07:30	10	0	15	1	0	0	3	2	0	18	1	34	1	1	52	3	1	0	11	0	0	0	0	0	0	0	3	143	6	7	149	
07:45	11	1	31	0	0	0	2	0	0	26	0	33	2	3	61	3	0	0	18	0	4	0	0	0	0	0	11	180	6	20	186	
08:00	14	0	46	0	0	0	6	1	0	54	2	19	4	2	54	3	1	0	22	1	4	0	0	1	0	3	0	20	215	10	32	225
08:15	21	0	64	11	0	0	12	1	0	80	3	16	1	5	52	3	4	0	41	0	3	0	0	3	0	18	0	30	300	18	50	318
08:30	23	0	50	1	0	0	20	1	0	50	1	28	3	3	51	3	1	0	32	1	3	0	0	1	0	5	0	30	242	9	56	251
08:45	17	2	51	3	0	0	9	1	0	55	2	34	1	12	54	2	2	0	34	1	4	1	0	0	0	0	13	249	11	38	260	
2 Hour Total	103	3	288	16	0	0	53	7	0	298	9	201	16	27	417	20	11	0	173	3	21	1	0	5	0	26	0	114	1530	67	215	1597
Peak Hour: 8:00 - 9:00	75	2	211	15	0	0	47	4	0	239	8	97	9	22	211	11	8	0	129	3	14	1	0	5	0	26	0	93	1006	48	176	1054
Total Veh & % Trucks	77	3%	226	7%	0	0%	4	0%	247	3%	106	8%	222	5%	8	0%	132	2%	1	0%	5	0%	26	0%	26	0%						
	PHF	1.26	Peak Total = 303						PHF	1.12	Peak Total = 357						PHF	1.10	Peak Total = 362						PHF	2.00	Peak Total = 32					

Time Starting	From the North On: Uxbridge Drive NW						From the South On: Uxbridge Drive NW						From the East On: Unwin Road NW						From the West On: Lane						Total			Total Vehicles						
	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Cars					
	Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars								
03:00	10	0	27	0	0	0	4	4	0	27	1	46	2	8	51	4	2	0	16	0	3	0	0	0	0	1	0	12	184	7	27	191		
03:15	13	0	45	3	0	0	14	3	0	46	0	43	1	38	58	3	0	0	15	1	0	0	0	8	0	13	0	82	244	8	134	252		
03:30	26	0	63	9	0	0	2	1	0	42	0	41	2	11	47	2	1	0	15	1	2	0	0	0	0	11	0	18	247	14	33	261		
03:45	15	1	35	1	0	0	6	1	0	33	0	54	1	5	49	2	0	0	11	1	9	0	0	0	0	5	0	10	203	6	30	209		
04:00	8	0	28	1	0	0	4	0	0	26	1	41	5	2	42	2	0	0	10	0	5	0	0	1	0	0	4	156	9	15	165			
04:15	8	0	25	0	0	0	1	0	0	21	0	61	3	5	45	2	0	0	13	0	3	0	0	0	0	2	0	6	175	5	15	180		
04:30	10	0	28	0	0	0	7	0	0	22	0	48	0	2	46	2	0	0	6	0	7	0	0	1	0	3	0	12	164	2	28	166		
04:45	6	0	37	0	0	0	3	1	0	22	0	56	1	1	60	1	0	0	10	0	4	0	0	0	0	0	5	192	2	13	194			
05:00	6	0	38	0	0	0	6	0	0	18	0	42	3	3	59	2	0	0	9	0	2	0	0	0	0	2	0	9	174	5	20	179		
05:15	6	0	26	0	0	0	3	1	0	18	0	47	2	0	44	1	0	0	7	0	0	1	0	0	0	2	0	6	152	3	9	155		
05:30	12	0	30	0	0	0	7	0	0	20	0	45	1	4	55	2	0	0	12	0	4	0	0	1	0	1	0	10	176	3	25	179		
05:45	13	0	15	0	0	0	5	1	0	24	0	55	3	2	49	1	1	0	13	0	2	0	0	1	0	2	0	10	174	4	19	178		
3 Hour Total	133	1	397	14	0	0	62	12	0	319	2	579	24	81	605	24	4	0	137	3	41	1	0	12	0	42	0	184	2241	68	368	2309		
Peak Hour: 3:00-4:00	64	1	170	13	0	0	26	9	0	148	1	184	6	62	205	11	3	0	57	3	14	0	0	8	0	30	0	122	878	35	224	913		
Total Veh & % Trucks	65	2%	183	7%	0	0%	9	0%	149	1%	190	3%	216	5%	3	0%	60	5%	0	0%	8	0%	30	0%	30	0%	2	17	68	0	298			
	PHF	1.41	Peak Total = 248						PHF	1.02	Peak Total = 348						PHF	0.98	Peak Total = 279						PHF	2.38	Peak Total = 38							

5 Hour Total	236	4	685	30	0	0	115	19	0	617	11	780	40	108	1022	44	15	0	310	6	62	2	0	17	0	68	0	298			
	240		715		0		19		628		820		1066		15		316		2		17		68		68						
	955						1467							1397										87							

Location : North Leg : University Drive  
 South Leg : University Drive  
 Date : Wednesday February 13, 2013  
 Observer(s) : Kelly  
 Job # : 2818.T02  
 Job Name: Stadium Shopping Centre TIA



Time Starting	From the North On: University Drive						From the South On: University Drive						From the East On: David Bauer Arena Access						From the West On: Unwin Road NW						Total			Total Vehicles								
	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Total							
	Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks									
07:00	2	0	34	4	43	2	1	10	0	71	3	0	0	6	1	0	0	0	0	0	1	16	0	1	0	7	0	4	185	9	12	194				
07:15	1	0	57	2	46	2	0	8	0	101	4	0	0	11	0	0	0	0	0	0	1	20	0	0	0	17	0	5	250	8	17	258				
07:30	3	0	86	5	57	2	3	10	0	152	4	1	0	25	0	0	0	0	0	0	1	38	2	0	0	14	0	8	361	13	37	374				
07:45	2	0	84	6	66	2	3	17	0	193	2	1	0	20	1	0	0	0	2	0	0	41	1	0	0	20	0	7	427	11	30	438				
08:00	2	0	87	2	63	4	5	26	6	180	3	2	0	14	1	0	1	0	0	0	0	32	2	0	0	7	0	11	401	17	30	418				
08:15	2	0	85	7	69	3	1	38	0	194	2	3	0	12	7	0	0	0	0	0	0	8	34	2	0	0	8	0	3	440	14	24	454			
08:30	1	0	72	2	59	4	0	27	1	220	3	2	0	35	2	0	1	0	2	0	0	3	42	3	0	0	15	0	0	443	13	38	456			
08:45	1	0	79	2	67	4	0	19	0	218	5	2	0	28	8	0	1	0	0	0	0	3	56	2	1	0	10	0	6	462	13	37	475			
2 Hour Total	14	0	584	30	470	23	13	155	7	1329	26	11	0	151	20	0	3	0	4	0	17	279	12	2	0	98	0	44	2969	98	225	3067				
Peak Hour: 8:00 - 9:00	6	0	323	13	258	15	6	110	7	812	13	9	0	89	18	0	3	0	2	0	0	14	164	9	1	0	40	0	20	1746	57	129	1803			
Total Veh & % Trucks	6	0%	336	4%	273	5%		117	6%	825	2%	9	0%		18	0%	3	0%	2	0%		173	5%	1	0%	40	0%									
	PHF	0.96	Peak Total = 615						PHF	1.10	Peak Total = 951						PHF	1.92	Peak Total = 23						PHF	0.86	Peak Total = 214									

Time Starting	From the North On: University Drive						From the South On: University Drive						From the East On: David Bauer Arena Access						From the West On: Unwin Road NW						Total			Total Vehicles								
	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Total							
	Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks									
03:00	6	0	131	2	47	2	48	27	1	99	1	3	0	8	2	0	0	0	0	0	0	63	1	2	0	11	0	0	391	7	56	398				
03:15	3	0	132	0	59	2	12	27	1	72	1	1	0	5	3	0	1	0	0	0	1	45	1	0	0	11	0	6	354	5	24	359				
03:30	1	0	123	2	48	1	11	22	0	108	4	5	1	6	1	0	1	0	0	0	0	68	0	0	0	21	0	5	398	8	22	406				
03:45	2	0	102	2	49	2	10	15	1	115	3	5	0	4	1	0	1	0	1	0	0	63	1	1	0	10	0	3	365	9	17	374				
04:00	4	0	117	2	45	1	34	31	0	107	1	1	0	2	5	0	1	0	0	0	1	43	3	3	0	7	0	5	364	7	42	371				
04:15	1	0	106	3	46	3	21	20	0	97	3	5	0	6	0	0	1	0	1	0	0	59	1	3	0	11	0	3	350	10	30	360				
04:30	6	0	109	4	46	1	12	9	0	110	1	5	0	4	1	0	0	0	0	0	0	48	0	0	0	13	0	3	347	6	19	353				
04:45	4	0	137	3	75	1	30	13	0	120	5	2	0	3	2	0	0	0	0	0	0	70	1	1	0	14	0	1	438	10	36	448				
05:00	2	0	150	3	57	2	27	27	0	133	0	4	0	3	2	0	3	0	0	0	0	49	2	0	0	11	0	1	438	7	31	445				
05:15	2	0	109	3	38	2	20	16	0	103	0	1	0	7	2	0	0	0	1	0	0	46	0	1	0	10	0	3	329	5	30	334				
05:30	2	0	87	2	47	1	19	25	0	101	2	1	0	1	0	0	0	0	0	0	0	51	0	0	0	8	0	7	322	5	27	327				
05:45	0	0	97	0	58	1	12	21	0	136	3	2	0	4	2	0	0	0	0	0	0	61	0	0	0	12	0	4	389	4	20	393				
3 Hour Total	33	0	1400	26	615	19	256	253	3	1301	24	35	1	53	21	0	8	0	3	0	4	666	10	11	0	139	0	41	4485	83	354	4568				
Peak Hour: 4:15 - 5:15	13	0	502	13	224	7	90	69	0	460	9	16	0	16	5	0	4	0	1	0	0	226	4	4	0	49	0	8	1573	33	116	1606				
Total Veh & % Trucks	13	0%	515	3%	231	3%		69	0%	469	2%	16	0%		5	0%	4	0%	1	0%		230	2%	4	0%	49	0%									
	PHF	0.86	Peak Total = 759						PHF	0.84	Peak Total = 554						PHF	0.42	Peak Total = 10						PHF	0.82	Peak Total = 283									

5 Hour Total	47	0	1984	56	1085	42	269	408	10	2630	50	46	1	204	41	0	11	0	7	0	21	945	22	13	0	237	0	85
	47		2040		1127		418		2680		47			41		11		7			967		13		237			
			3214				3145							59							1217							

Location : North Leg : Uxbridge Drive NW East Leg:  
South Leg : Uxbridge Drive NW West Leg: North Mall Acces  
Date : Thursday February 21, 2013  
Observer(s) : Matthew **NOTE: through movement**  
Job # : 2818.T02 **counted**  
Job Name: Stadium Shopping Centre TIA



Time Starting	From the North On: Uxbridge Drive NW						From the South On: Uxbridge Drive NW						From the East On:						From the West On: North Mall Access						Total			Total Vehicles							
	Left Turn		Straight		Right Turn		Ped	Left Turn		Straight		Right Turn		Ped	Left Turn		Straight		Right Turn		Ped	Left Turn		Straight		Right Turn		Ped	Cars			Total Vehicles			
	Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars						
03:00					9			15																			18			8		50	0	0	50
03:15					13			18																			8			25		64	0	0	64
03:30					19			26																			5			31		81	0	0	81
03:45					15			8																			14			13		50	0	0	50
04:00					25			6																			6			19		56	0	0	56
04:15					13			8																			7			16		44	0	0	44
04:30					14			21																			12			17		64	0	0	64
04:45					19			16																			11			23		69	0	0	69
05:00					13			17																			14			22		66	0	0	66
05:15					10			13																			13			14		50	0	0	50
05:30					12			13																			6			12		43	0	0	43
05:45					12			13																			16			16		57	0	0	57
3 Hour Total	0	0	0	0	174	0	0	174	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	130	0	0	0	216	0	0	694		
Peak Hour: 3:15-4:15	0	0	0	0	72	0	0	58	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	33	0	0	0	88	0	0	245		
Total Veh & % Trucks	0	0%	0	0%	72	0%		58	0%	0	0%	0	0%		0	0%	0	0%	0	0%		33	0%	0	0%	88	0%								
	PHF	0.72			Peak Total =	72		PHF	0.69			Peak Total =	58		PHF	0.00			Peak Total =	0		PHF	0.84			Peak Total =	121								

5 Hour Total	0	0	0	0	220	0	0	239	0	0	0	0	0	0	0	0	0	0	143	0	0	0	273	0	0
	0	0	220		239	0	0	0	0	0	0	0	0	0	0	0	0	143	0	0	273				
220												239												416	

Location : North Leg : Uxbridge Drive NW  
South Leg : Uxbridge Drive NW  
Date : Thursday February 21, 2013  
Observer(s) : Kelly  
Job # : 2818.T02  
Job Name: Stadium Shopping Centre T

East Leg:

**NOTE: through movements on Uxbridge Drive not counted**



Time Starting	From the North On: Uxbridge Drive NW						From the South On: Uxbridge Drive NW						From the East On:						From the West On: South Mall Access						Total			Total Vehicles					
	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Cars			Total Vehicles	
	Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars				
07:00					0			12														0					7			19	0	0	19
07:15					0			14														0					2			16	0	0	16
07:30					0			9														1					7			17	0	0	17
07:45					1			16														0					9			26	0	0	26
08:00					1			14														0					9			24	0	0	24
08:15					3			15														0					14			32	0	0	32
08:30					2			17														0					13			32	0	0	32
08:45					0			16														3					10			29	0	0	29
2 Hour Total	0	0	0	0	7	0	0	113	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	0	71	0	0	195	0	0	195	
Peak Hour: 8:00 - 9:00	0	0	0	0	6	0	0	62	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	0	46	0	0	117	0	0	117		
Total Veh & % Trucks	0	0%	0	0%	6	0%		62	0%	0	0%	0	0%		0	0%	0	0%	0	0%	3	0%	0	0%	46	0%							
PHF	1.50				Peak Total =	6	PHF	0.97				Peak Total =	62	PHF	0.00				Peak Total =	0	PHF	1.36				Peak Total =	49						

Time Starting	From the North On: Uxbridge Drive NW						From the South On: Uxbridge Drive NW						From the East On:						From the West On: South Mall Access						Total			Total Vehicles						
	Left Turn		Straight		Right Turn		Ped	Left Turn		Straight		Right Turn		Ped	Left Turn		Straight		Right Turn		Ped	Left Turn		Straight		Right Turn		Ped	Cars			Total Vehicles		
	Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars					
03:00					4			37														1					30				72	0	0	72
03:15					1			40														3					42				86	0	0	86
03:30					7			30														1					41				79	0	0	79
03:45					1			25														1					21				48	0	0	48
04:00					2			36														0					40				78	0	0	78
04:15					1			37														3					29				70	0	0	70
04:30					4			38														0					33				75	0	0	75
04:45					7			23														2					39				71	0	0	71
05:00					5			43														0					34				82	0	0	82
05:15					8			43														2					35				88	0	0	88
05:30					6			36														5					31				78	0	0	78
05:45					5			37														3					29				74	0	0	74
3 Hour Total	0	0	0	0	51	0	0	425	0	0	0	0	0	0	0	0	0	0	0	0	0	21	0	0	0	404	0	0	901	0	0	901		
Peak Hour: 5:00-6:00	0	0	0	0	24	0	0	159	0	0	0	0	0	0	0	0	0	0	0	0	10	0	0	0	129	0	0	285	0	0	285			
Total Veh & % Trucks	0	0%	0	0%	24	0%		159	0%	0	0%	0	0%		0	0%	0	0%	0	0%	10	0%	0	0%	129	0%								
	PHF	0.86			Peak Total =	24		PHF	0.92			Peak Total =	159		PHF	0.00			Peak Total =	0		PHF	0.85			Peak Total =	139							

5 Hour Total	0	0	0	0	58	0	0	538	0	0	0	0	0	0	0	0	0	0	0	25	0	0	0	475	0	0
	0	0	58		538	0	0	0	0	0	0	0	0	0	0	0	0	0	0	25	0	0	0	475		
	58				538			0											500							

Location : North Leg : Uxbridge Drive NW  
 South Leg : Uxbridge Drive NW  
 Date : Tuesday February 12, 2013  
 Observer(s) : Kelly  
 Job # : 2818.T02  
 Job Name: Stadium Shopping Centre TIA



Time Starting	From the North On: Uxbridge Drive NW						From the South On: Uxbridge Drive NW						From the East On: Ulster Road NW						From the West On:						Total			Total Vehicles				
	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Total			Total Vehicles
	Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks					
07:00	1	0	75	2	0	0	4	0	0	38	3	7	0	2	8	0	0	0	1	0	4	0	0	0	0	0	0	0	130	5	10	135
07:15	0	0	56	0	0	0	4	0	0	44	1	2	0	2	6	0	0	0	1	0	4	0	0	0	0	0	0	0	109	1	10	110
07:30	0	0	82	3	0	0	5	0	0	36	3	3	0	2	7	0	0	0	0	0	2	0	0	0	0	0	0	0	128	6	9	134
07:45	1	1	74	1	0	0	6	0	0	60	1	7	0	6	2	0	0	0	1	0	8	0	0	0	0	0	0	2	145	3	22	148
08:00	1	0	92	4	0	0	8	0	0	71	7	6	0	10	3	0	0	0	1	0	10	0	0	0	0	0	0	0	174	11	28	185
08:15	2	0	136	14	0	0	20	0	0	97	3	2	0	6	2	0	0	0	3	0	8	0	0	0	0	0	0	0	242	17	34	259
08:30	0	0	118	3	0	0	26	0	0	85	5	2	0	9	9	0	0	0	2	0	19	0	0	0	0	0	0	4	216	8	58	224
08:45	2	0	77	3	0	0	10	0	0	75	3	13	0	11	0	0	0	0	3	0	30	0	0	0	0	0	0	15	170	6	66	176
2 Hour Total	7	1	710	30	0	0	83	0	0	506	26	42	0	48	37	0	0	0	12	0	85	0	0	0	0	0	0	21	1314	57	237	1371
Peak Hour: 8:00 - 9:00	5	0	423	24	0	0	64	0	0	328	18	23	0	36	14	0	0	0	9	0	67	0	0	0	0	0	0	19	802	42	186	844
Total Veh & % Trucks	5	0%	447	5%	0	0%		0	0%	346	5%	23	0%	14	0%	0	0%	9	0%	0	0%	0	0%	0	0%	0	0%					
	PHF	1.16	Peak Total = 452						PHF	1.10	Peak Total = 369						PHF	0.64	Peak Total = 23						PHF	0.00	Peak Total = 0					

Time Starting	From the North On: Uxbridge Drive NW						From the South On: Uxbridge Drive NW						From the East On: Ulster Road NW						From the West On:						Total			Total Vehicles				
	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Total			Total Vehicles
	Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks					
03:00	1	0	58	1	0	0	6	0	0	77	2	12	0	17	5	0	0	0	0	0	5	0	0	0	0	0	0	4	153	3	32	156
03:15	0	0	92	2	0	0	19	0	0	97	1	12	0	15	5	0	0	0	6	0	9	0	0	0	0	0	0	0	212	3	43	215
03:30	1	0	147	4	0	0	10	0	0	93	2	4	0	12	6	0	0	0	1	0	8	0	0	0	0	0	0	0	252	6	30	258
03:45	0	0	72	1	0	0	9	0	0	100	2	5	0	17	5	0	0	0	2	0	5	0	0	0	0	0	0	1	184	3	32	187
04:00	5	0	82	1	0	0	3	0	0	76	6	3	0	19	5	0	0	0	0	0	4	0	0	0	0	0	0	0	171	7	26	178
04:15	1	0	94	0	0	0	13	0	0	86	0	9	0	10	4	0	0	0	3	0	7	0	0	0	0	0	0	2	197	0	32	197
04:30	1	0	56	2	0	0	5	0	0	69	0	7	0	17	8	0	0	0	0	0	3	0	0	0	0	0	0	5	141	2	30	143
04:45	0	0	80	0	0	0	2	0	0	69	0	3	0	11	11	0	0	0	1	0	9	0	0	0	0	0	0	10	164	0	32	164
05:00	0	0	84	0	0	0	6	0	0	64	0	8	0	14	7	0	0	0	1	0	4	0	0	0	0	0	0	7	164	0	31	164
05:15	3	0	81	0	0	0	2	0	0	64	1	4	0	16	7	0	0	0	1	0	11	0	0	0	0	0	0	9	160	1	38	161
05:30	0	0	68	0	0	0	6	0	0	58	0	13	0	9	7	0	0	0	0	0	8	0	0	0	0	0	0	3	146	0	26	146
05:45	3	0	46	0	0	0	3	0	0	53	0	11	0	10	1	0	0	0	2	0	7	0	0	0	0	0	0	5	116	0	25	116
3 Hour Total	15	0	960	11	0	0	84	0	0	906	14	91	0	167	71	0	0	0	17	0	80	0	0	0	0	0	0	46	2060	25	377	2085
Peak Hour: 3:15-4:15	6	0	393	8	0	0	41	0	0	366	11	24	0	63	21	0	0	0	9	0	26	0	0	0	0	0	0	1	819	19	131	838
Total Veh & % Trucks	6	0%	401	2%	0	0%		0	0%	377	3%	24	0%	21	0%	0	0%	9	0%	0	0%	0	0%	0	0%	0	0%					
	PHF	1.07	Peak Total = 407						PHF	1.06	Peak Total = 401						PHF	0.63	Peak Total = 30						PHF	0.00	Peak Total = 0					

5 Hour Total	22	1	1670	41	0	0	167	0	0	1412	40	133	0	215	108	0	0	0	29	0	165	0	0	0	0	0	67				
	23		1711		0		0		1452		133		108		0		29		0	0	0	0		0		0					
		1734			1585					137														0							

Location : North Leg : Uxbridge Drive NW  
 South Leg : 29 Street NW  
 Date : Tuesday February 12, 2013  
 Observer(s) : Roland & Barb  
 Job # : 2818.T02  
 Job Name: Stadium Shopping Centre TIA



Time Starting	From the North On: Uxbridge Drive NW							From the South On: 29 Street NW							From the East On: 16 Avenue NW							From the West On: 16 Avenue NW							Total			Total Vehicles						
	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Cars			Total Vehicles						
	Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars			Total Vehicles						
07:00	25	0	76	2	12	1	2	19	0	38	1	44	2	5	84	4	121	9	23	1	14	16	0	295	1	103	2	0	856	23	21	879						
07:15	23	0	57	1	20	2	0	45	1	36	1	67	2	7	93	4	110	12	14	2	5	17	0	385	3	131	1	18	998	29	30	1027						
07:30	25	0	64	4	21	0	5	33	0	28	2	40	2	7	90	2	138	13	46	0	8	30	1	391	8	147	2	15	1053	34	35	1087						
07:45	20	0	66	2	15	1	5	43	2	35	2	51	1	18	111	3	189	15	21	0	7	38	1	331	8	139	2	28	1059	37	58	1096						
08:00	23	2	80	3	19	0	20	20	1	36	2	30	4	14	88	5	118	12	30	1	19	36	3	338	4	163	3	36	981	40	89	1021						
08:15	33	1	102	14	35	0	25	28	2	45	3	41	2	23	99	4	188	14	36	0	10	51	0	280	4	81	0	59	1019	44	117	1063						
08:30	24	0	100	3	34	1	9	19	1	43	2	62	1	18	105	1	204	7	32	2	12	50	0	292	7	120	3	62	1085	28	101	1113						
08:45	20	1	75	4	31	0	20	31	3	36	2	53	3	29	97	1	161	18	31	1	34	25	2	252	7	87	4	46	899	46	129	945						
2 Hour Total	193	4	620	33	187	5	86	238	10	297	15	388	17	121	767	24	1229	100	233	7	109	263	7	2564	42	971	17	264	7950	281	580	8231						
Peak Hour: 7:45 - 8:45	100	3	348	22	103	2	59	110	6	159	9	184	8	73	403	13	699	48	119	3	48	175	4	1241	23	503	8	185	4144	149	365	4293						
Total Veh & % Trucks	103	3%	370	6%	105	2%		116	5%	168	5%	192	4%		416	3%	747	6%	122	2%		179	2%	1264	2%	511	2%											
	PHF	1.14						Peak Total =	578	PHF	0.78				Peak Total =	476	PHF	0.95				Peak Total =	1285	PHF	0.84			Peak Total =	1954									

Time Starting	From the North On: Uxbridge Drive NW							From the South On: 29 Street NW							From the East On: 16 Avenue NW							From the West On: 16 Avenue NW							Total			Total Vehicles
	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Left Turn		Straight		Right Turn		Peds	Cars			Total Vehicles
	Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars	Trucks	Cars	Trucks	Cars	Trucks		Cars			Total Vehicles
03:00	27	0	32	3	29	0	5	89	0	80	3	149	1	15	81	2	218	12	39	3	20	26	0	186	5	23	1	37	979	30	77	1009
03:15	43	2	68	4	66	1	2	98	2	71	2	146	3	24	56	2	204	11	36	0	8	29	0	177	11	32	2	19	1026	40	53	1066
03:30	37	0	57	13	72	1	1	91	1	80	3	141	3	22	54	5	239	3	14	1	11	26	0	219	12	22	3	41	1052	45	75	1097
03:45	46	0	42	4	50	0	1	115	0	85	2	142	4	20	65	5	236	5	34	2	32	29	0	210	9	23	0	20	1077	31	73	1108
04:00	32	1	48	2	55	0	9	102	3	68	3	149	2	23	53	6	270	7	36	1	15	21	1	221	7	17	0	16	1072	33	63	1105
04:15	24	1	42	3	59	1	2	101	0	80	3	112	4	20	47	2	284	7	40	0	16	21	0	213	7	35	1	27	1058	29	65	1087
04:30	22	1	25	3	63	0	4	119	2	64	3	104	1	19	53	2	314	4	27	1	4	32	0	195	7	16	2	13	1034	26	40	1060
04:45	35	0	50	1	57	1	3	83	1	52	0	107	1	19	55	2	309	3	36	0	27	30	0	202	11	25	2	35	1041	22	84	1063
05:00	25	0	40	1	59	1	5	83	0	46	3	83	4	18	43	2	336	5	34	1	15	36	1	178	6	22	1	28	985	25	66	1010
05:15	27	0	39	2	53	0	10	74	2	46	3	68	3	30	51	1	305	2	32	0	24	32	1	232	0	27	2	27	986	16	91	1002
05:30	30	0	31	2	40	0	7	52	1	56	1	70	1	22	49	2	275	6	31	1	15	21	1	196	10	22	1	29	873	26	73	899
05:45	20	0	43	0	41	1	5	60	1	48	3	55	2	16	46	2	274	1	42	0	11	33	0	191	2	27	2	30	880	14	62	894
3 Hour Total	368	5	517	38	644	6	54	1067	13	776	29	1326	29	248	653	33	3264	66	401	10	198	336	4	2420	87	291	17	322	12063	337	822	12400
Peak Hour: 3:30-4:30	139	2	189	22	236	2	13	409	4	313	11	544	13	85	219	18	1029	22	124	4	74	97	1	863	35	97	4	104	4259	138	276	4397
Total Veh & % Trucks	141	1%	211	10%	238	1%		413	1%	324	3%	557	2%		237	8%	1051	2%	128	3%		98	1%	898	4%	101	4%					
	PHF	1.02						Peak Total =	590	PHF	0.99				Peak Total =	1294	PHF	0.84				Peak Total =	1416	PHF	0.99			Peak Total =	1097			

5 Hour Total	561	9	1137	71	831	11	140	1305	23	1073	44	1714	46	369	1420	57	4493	166	634	17	307	599	11	4984	129	1262	34	586				
	570		1208		842			1328		1117		1760			1477		4659		651			610		5113		1296						
	2620													4205													6787			7019		

## APPENDIX C: MODE SPLIT COUNTS

Stadium Shopping Centre  
Mode Split Count - Raw Data

Time Start	Shopping Centre												Peak Hour	
	Vehicles		Passengers		Pedestrians		Bicycles		Transit		Total			
	In	Out	In	Out	In	Out	In	Out	In	Out	In	Out		
07:00	17	13	0	3	3	5	1	0	0	0	21	21		
07:15	19	5	0	1	2	10	1	1	0	0	22	17		
07:30	19	14	1	6	2	12	0	0	0	0	22	32		
07:45	28	15	2	3	3	11	0	1	0	0	33	30		
08:00	29	15	2	1	4	10	0	0	0	0	35	26		
08:15	45	39	7	8	3	4	0	0	0	0	55	51		
08:30	51	33	11	6	19	25	2	1	1	0	84	65		
08:45	40	21	2	1	30	31	1	1	1	0	74	54		
Total	248	155	25	29	66	108	5	4	2	0	346	296		
Peak Hour Total	165	108	22	16	56	70	3	2	2	0	248	196		
MODE SPLIT	72%	52%	7%	10%	19%	36%	1%	1%	1%	0%				

Peak Hour

444 8:00-9:00

	Vehicles		Passenger		Pedestrians		Bicycles		Transit		Total		Peak Hour
	In	Out	In	Out	In	Out	In	Out	In	Out	In	Out	
	66	58	1	1	21	16	0	0	0	1	88	76	
03:00	66	58	1	1	21	16	0	0	0	1	88	76	
03:15	80	81	9	12	44	18	0	0	0	0	133	111	
03:30	84	79	6	18	31	26	1	0	0	0	122	123	
03:45	49	49	8	10	25	15	0	0	0	2	82	76	
04:00	69	65	12	6	18	21	0	0	0	1	99	93	
04:15	59	56	7	5	21	18	0	0	0	0	87	79	
04:30	77	63	10	2	20	13	1	0	0	0	108	78	
04:45	65	75	13	8	22	14	0	0	0	0	100	97	
05:00	79	70	7	3	17	7	0	0	0	3	103	83	
05:15	74	67	6	3	13	13	0	0	0	1	93	84	
05:30	67	55	8	5	24	8	0	1	0	0	99	69	
05:45	67	65	4	10	20	6	1	0	0	0	92	81	
Total	836	783	91	83	276	175	3	1	0	8	1206	1050	
Peak Hour Total	282	274	35	46	118	80	1	0	0	3	436	403	
MODE SPLIT	69%	75%	8%	8%	23%	17%	0%	0%	0%	1%			

Peak Hour

839 3:15-4:15

**NOTE:** Due to the large number of movements observed, occupancy was not monitored for vehicle lane Access

Assumed to be one

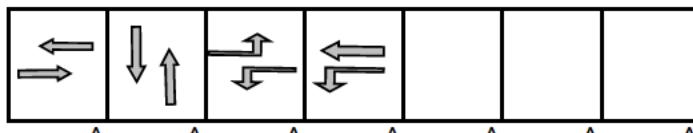
## APPENDIX D: SIGNAL TIMING PLANS



## SIGNAL TIMING SUMMARY

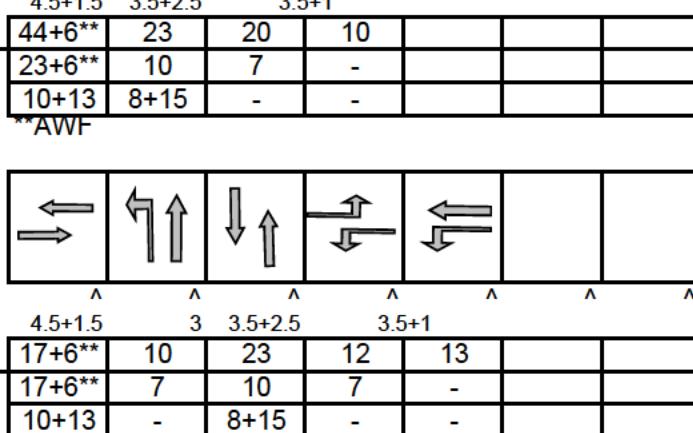
**LOCATION:**
16 AVENUE - UXBRIDGE DR/29 STREET NW
**INT #:**
191
**Date Coded:**
2012 NOV 29
**Date Installed:**

**TIMING PLAN NO:** MP1  
**CYCLE LENGTH:** max 119.5  
**OFFSET:** -  
**START TIME:** 6:30  
**END TIME:** 9:00



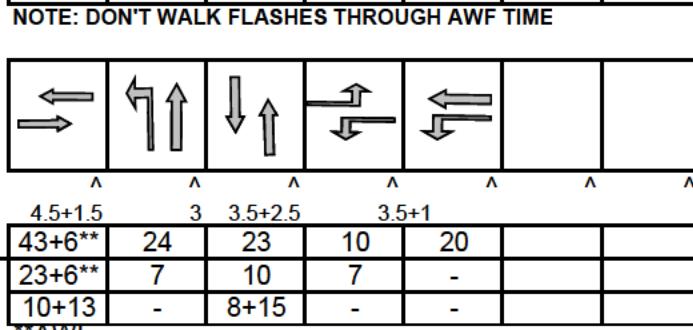
	Pro Only	Pro/Per	Per Only
NBLT			x
SBLT			x
EBLT	x		
WBLT	x		

**TIMING PLAN NO:** MP2  
**CYCLE LENGTH:** max 100.5  
**OFFSET:** -  
**START TIME:** 9:00  
**END TIME:** 15:30

**WEEKDAYS** 18:30 - 23:00


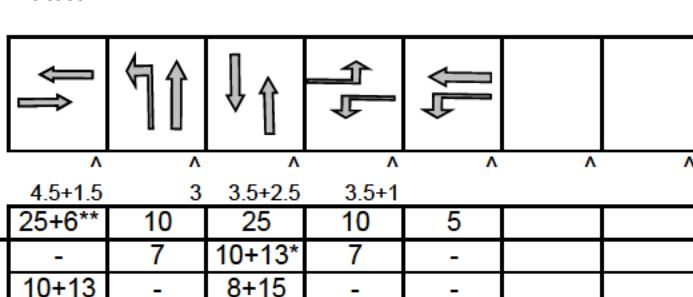
	Pro Only	Pro/Per	Per Only
NBLT		x	
SBLT			x
EBLT	x		
WBLT	x		

**TIMING PLAN NO:** MP3  
**CYCLE LENGTH:** max 143.5  
**OFFSET:** -  
**START TIME:** 15:30  
**END TIME:** 18:30



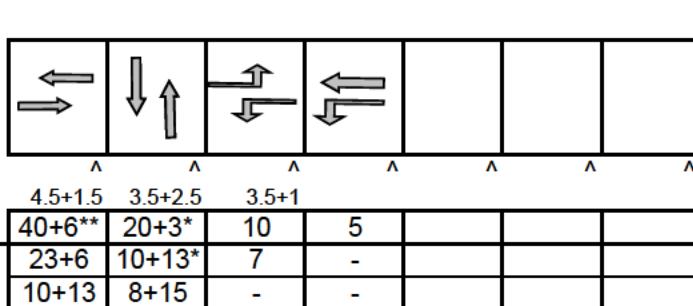
	Pro Only	Pro/Per	Per Only
NBLT		x	
SBLT			x
EBLT	x		
WBLT	x		

**TIMING PLAN NO:** MP4  
**CYCLE LENGTH:** max 100.5  
**OFFSET:** -  
**START TIME:** 6:30  
**END TIME:** 23:00

**SAT & SUN**


	Pro Only	Pro/Per	Per Only
NBLT		x	
SBLT			x
EBLT	x		
WBLT	x		

**TIMING PLAN NO:** FREE  
**CYCLE LENGTH:** max 100.5  
**OFFSET:** -  
**START TIME:** 23:00  
**END TIME:** 6:30

**SAT & SUN** 23:00 - 6:30


	Pro Only	Pro/Per	Per Only
NBLT			x
SBLT			x
EBLT	x		
WBLT	x		

\*If pedestrian-actuated, \*\* Advance warning flash

Notes: The offset point is referenced to the beginning of the first column of traffic movements.

If the max time is less than the pedestrian time, the extra unused pedestrian time is passed to the main street unless otherwise noted.



## SIGNAL TIMING SUMMARY

LOCATION:

University Drive - Unwin Road NW

INT #: 179

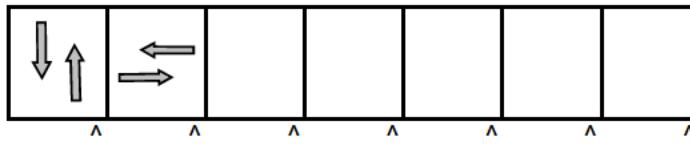
Date Coded:

Dec 28 2010

Date Installed:

Removed:

TIMING PLAN NO: MAX I  
CYCLE LENGTH: max 71.5  
OFFSET:  
START TIME: 06:00  
END TIME: 24:00



	Pro Only	Pro/Per	Per Only
NBLT			x
SBLT			x
EBLT			x
WBLT			x

FLASH 00:00-06:00

4+1.5 3.5+2.5

MAX  
MIN if Actuated  
Pedestrian

MAX	35	25					
MIN if Actuated	21	10+13*					
Pedestrian	8+13	8+15					

with ped

TIMING PLAN NO:  
CYCLE LENGTH:  
OFFSET:  
START TIME:  
END TIME:

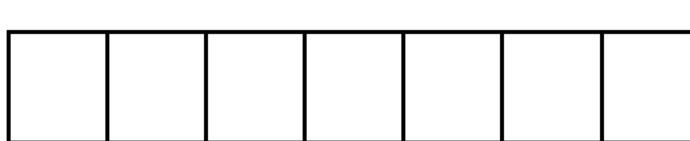


	Pro Only	Pro/Per	Per Only
NBLT			
SBLT			
EBLT			
WBLT			

MAX  
MIN if Actuated  
Pedestrian

MAX							
MIN if Actuated							
Pedestrian							

TIMING PLAN NO:  
CYCLE LENGTH:  
OFFSET:  
START TIME:  
END TIME:



	Pro Only	Pro/Per	Per Only
NBLT			
SBLT			
EBLT			
WBLT			

MAX  
MIN if Actuated  
Pedestrian

MAX							
MIN if Actuated							
Pedestrian							

TIMING PLAN NO:  
CYCLE LENGTH:  
OFFSET:  
START TIME:  
END TIME:



	Pro Only	Pro/Per	Per Only
NBLT			
SBLT			
EBLT			
WBLT			

MAX  
MIN if Actuated  
Pedestrian

MAX							
MIN if Actuated							
Pedestrian							

TIMING PLAN NO:  
CYCLE LENGTH:  
OFFSET:  
START TIME:  
END TIME:



	Pro Only	Pro/Per	Per Only
NBLT			
SBLT			
EBLT			
WBLT			

MAX  
MIN if Actuated  
Pedestrian

MAX							
MIN if Actuated							
Pedestrian							

Notes: The offset point is referenced to the beginning of the first column of traffic movements.

## APPENDIX E: SYNCHRO ANALYSIS RESULTS

HCM Unsignalized Intersection Capacity Analysis  
5: Uxbridge Drive & Lane

Existing Conditions  
AM Peak Hour

Movement	EBT	EBC	WBL	WBT	NBL	NBR
Lane Configurations	→	↔	↔	↔	↑	↓
Volume (veh/h)	254	4	140	251	1	60
Sign Control	Free	Free	Stop	Stop	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	270	4	149	267	1	64
Pedestrians	1		23	54		
Lane Width (m)	4.3		4.3	3.5		
Walking Speed (m/s)	1.2		1.2	1.2		
Percent Blockage	0		2	4		
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume			328		892	349
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			328		892	349
IC, single (s)			4.1		6.4	6.2
IC, 2 stage (s)						
If (s)			2.2		3.5	3.3
p0 queue free %			87		100	90
cM capacity (veh/h)			1177		261	648
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	274	416	65			
Volume Left	0	149	1			
Volume Right	4	0	64			
cSH	1700	1177	633			
Volume to Capacity	0.16	0.13	0.10			
Queue Length 95th (m)	0.0	3.5	2.7			
Control Delay (s)	0.0	3.9	11.3			
Lane LOS	A	B				
Approach Delay (s)	0.0	3.9	11.3			
Approach LOS			B			
Intersection Summary						
Average Delay			3.1			
Intersection Capacity Utilization			55.5%	ICU Level of Service	B	
Analysis Period (min)			15			

Stadium Shopping Centre  
TIA

Synchro 7 - Report  
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HCM Unsignalized Intersection Capacity Analysis  
1: Unwin Road & Uxbridge Drive

Existing Conditions  
AM Peak Hour

Movement	EBL	EBC	WBL	WBT	NBL	NBR	SBL	SBT	SBR
Lane Configurations	↑	↔	↔	↔	↑	↓	↑	↔	↔
Sign Control	Stop	Free	Stop	Stop	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	1	5	26	242	8	143	4	247	121
Pedestrians	1	5	28	152	129	0			
Lane Width (m)	4.3		4.3	3.5					
Walking Speed (m/s)	1.2		1.2	1.2					
Percent Blockage	0		2	4					
Right turn flare (veh)									
Median type	None		None						
Median storage (veh)									
Upstream signal (m)									
pX, platoon unblocked									
vC, conflicting volume			328		892	349			
vC1, stage 1 conf vol									
vC2, stage 2 conf vol									
vCu, unblocked vol			328		892	349			
IC, single (s)			4.1		6.4	6.2			
IC, 2 stage (s)									
If (s)			2.2		3.5	3.3			
p0 queue free %			87		100	90			
cM capacity (veh/h)			1177		261	648			
Direction, Lane #	EB 1	WB 1	NB 1						
Volume Total	34	418	396		334				
Volume Left	1	257	4		94				
Volume Right	28	152	129		0				
Head (s)	0.45	0.03	-0.11		0.16				
Departure Headway (s)	6.8	6.1	6.0		6.3				
Degree Utilization, x	0.06	0.71	0.65		0.59				
Capacity (veh/h)	397	561	574		531				
Control Delay (s)	10.3	22.7	19.5		17.9				
Approach Delay (s)	10.3	22.7	19.5		17.9				
Approach LOS	B	C	C		C				
Intersection Summary									
Average Delay			19.9						
HCM Level of Service			C						
Intersection Capacity Utilization			80.1%	ICU Level of Service	D				
Analysis Period (min)			15						

Stadium Shopping Centre

Synchro 7 - Report  
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HCM Unsignalized Intersection Capacity Analysis  
14: North Driveway & Uxbridge Drive

Existing Conditions  
AM Peak Hour

Movement	EBL	EBC	NBL	NBT	SBT	SBR
Lane Configurations	↑	↔	↔	↔	↑	↔
Volume (veh/h)	9	41	50	363	460	34
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	10	44	53	386	489	36
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (m)			221			
pX, platoon unblocked	0.99					
vC, conflicting volume	1000	507	526			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	996	507	526			
IC, single (s)	6.4	6.2	4.1			
IC, 2 stage (s)						
If (s)	3.5	3.3	2.2			
p0 queue free %	96	92	95			
cM capacity (veh/h)	255	565	1041			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	53	439	526			
Volume Left	10	53	0			
Volume Right	44	0	36			
cSH	464	1041	1700			
Volume to Capacity	0.11	0.05	0.31			
Queue Length 95th (m)	3.1	1.3	0.0			
Control Delay (s)	13.8	1.5	0.0			
Lane LOS	B	A				
Approach Delay (s)	13.8	1.5	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay			1.4			
Intersection Capacity Utilization			62.8%	ICU Level of Service	B	
Analysis Period (min)			15			

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HCM Unsignalized Intersection Capacity Analysis  
16: Ulster Road & Uxbridge Drive

Existing Conditions  
AM Peak Hour

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↔	↑	↔	↑	↔
Volume (veh/h)	14	9	404	23	5	496
Sign Control	Stop	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	15	10	430	24	5	528
Pedestrians	67		36			64
Lane Width (m)	3.5		3.5			4.8
Walking Speed (m/s)	1.2		1.2			1.2
Percent Blockage	5		3			7
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (m)			153			
pX, platoon unblocked						
vC, conflicting volume			1083	358		521
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			1083	358		521
IC, single (s)	6.8	6.9				4.1
IC, 2 stage (s)						
If (s)	3.5	3.3				2.2
p0 queue free %	92	98				99
cM capacity (veh/h)	193	561				985
Direction, Lane #	WB 1	NB 1	NB 2	SB 1		
Volume Total	24	287	168	533		
Volume Left	15	0	0	5		
Volume Right	10	0	24	0		
cSH	260	1700	1700	985		
Volume to Capacity	0.09	0.17	0.10	0.01		
Queue Length 95th (m)	2.5	0.0	0.0	0.1		
Control Delay (s)	20.3	0.0	0.0	0.2		
Lane LOS	C		A			
Approach Delay (s)	20.3	0.0		0.2		
Approach LOS	C					
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utilization			49.7%	ICU Level of Service	A	
Analysis Period (min)			15			

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HCM Unsignalized Intersection Capacity Analysis  
12: South Driveway & Uxbridge Drive

Existing Conditions  
AM Peak Hour

Movement	EBL	EBC	NBL	NBT	SBT	SBR
Lane Configurations						
Volume (veh/h)	3	46	62	424	504	6
Sign Control	Stop	Free	Free			
Grade	0%	0%	0%			
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	
Hourly flow rate (vph)	3	49	66	451	536	6
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None	None		
Median storage (veh)						
Upstream signal (m)			94			
VC, conflicting volume	897	271	543			
vc1, stage 1 conf vol						
vc2, stage 2 conf vol						
vcu, unblocked vol	897	271	543			
IC, single (s)	6.8	6.9	4.1			
IC, 2 stage (s)						
tf (s)	3.5	3.3	2.2			
p0 queue free %	99	93	94			
CM capacity (veh/h)	261	727	1022			
Direction, Lane #	EB1	NB1	NB2	NB3	SB1	SB2
Volume Total	52	66	226	226	357	185
Volume Left	3	66	0	0	0	0
Volume Right	49	0	0	0	0	6
CSH	655	1022	1700	1700	1700	1700
Volume to Capacity	0.08	0.06	0.13	0.13	0.21	0.11
Queue Length 95th (m)	2.1	1.7	0.0	0.0	0.0	0.0
Control Delay (s)	11.0	8.8	0.0	0.0	0.0	0.0
Lane LOS	B	A				
Approach Delay (s)	11.0	1.1			0.0	
Approach LOS	B					
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utilization	31.4%		ICU Level of Service		A	
Analysis Period (min)	15					

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Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

Existing Conditions  
AM Peak Hour

Lane Group	EBL	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations											
Volume (vph)	179	1264	511	416	747	122	116	168	192	103	370
Ideal Flow (vphp)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5
Storage Length (m)	85.0		0.0	75.0		0.0	110.0		0.0	50.0	0.0
Storage Lanes	1		1	1		1	1		0	1	0
Taper Length (m)	7.5		7.5	7.5		7.5	7.5		7.5	7.5	7.5
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	0.95	1.00	0.95
Ped Bike Factor	0.97		0.88	0.98		0.90	0.89	0.95	0.96	0.93	
Frt			0.850			0.850		0.920		0.967	
Flt Protected	0.950				0.950				0.950		
Satd. Flow (prot)	1704	3408	1649	1687	3279	1691	1655	2915	0	1687	2988
Flt Permitted	0.950				0.950			0.251		0.387	
Satd. Flow (perm)	1652	3408	1446	1658	3279	1519	389	2915	0	661	2988
Right Turn on Red			Yes			Yes		Yes		Yes	Yes
Satd. Flow (RTOR)			58			130			204		27
Link Speed (kph)	70				70			50		50	
Link Distance (m)	328.0				201.4			258.5		93.8	
Travel Time (s)	16.9				10.4			18.6		6.8	
Conf. Peds. (#/hr)	59		73	73		59	185		48	48	185
Conf. Bikes (#/hr)									4		3
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	2%	2%	2%	3%	6%	2%	5%	5%	4%	3%	2%
Bus Blockages (#/hr)	0	0	14	0	0	8	0	0	0	0	0
Adj. Flow (vph)	190	1345	544	443	795	130	123	179	204	110	394
Shared Lane Traffic (%)											
Lane Group Flow (vph)	190	1345	544	443	795	130	123	383	0	110	506
Enter Blocked Intersection	No	No									
Lane Alignment	Left	Right	Left	Left	Right	Left	Left	Right	Left	Right	
Median Width(m)	3.5				3.5				3.5		
Link Offset(m)	0.0				0.0			0.0		0.0	
Crosswalk Width(m)	4.8				4.8			4.8		4.8	
Two way Left Turn Lane											
Headway Factor	1.05	1.05	0.95	1.05	1.05	0.92	1.05	1.05	0.88	1.05	1.05
Turning Speed (kph)	25		15	25		15	25		15	25	15
Number of Detectors	1	1	1	1	1	1	1	1	1	1	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Left	Thru	
Leading Detector (m)	8.0	4.0	4.0	8.0	4.0	2.0	8.0	4.0	8.0	4.0	
Trailing Detector (m)	2.0	2.0	2.0	2.0	2.0	0.0	2.0	2.0	2.0	2.0	
Detector 1 Position(m)	2.0	2.0	2.0	2.0	2.0	0.0	2.0	2.0	2.0	2.0	
Detector 1 Size(m)	6.0	2.0	2.0	6.0	2.0	2.0	6.0	2.0	6.0	2.0	
Detector 1 Type	Cl+Ex										
Detector 1 Channel											
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Turn Type	Prot		Perm	Prot		Perm	Perm	Perm		Perm	
Protected Phases	5	2		1	6		6	8		8	4
Permitted Phases				2			6	8			

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Lanes, Volumes, Timings 7: 16 Avenue & Uxbridge Drive											
Existing Conditions AM Peak Hour											
Lane Group											
Detector Phase											
Switch Phase											
Minimum Initial (s)											
Minimum Split (s)											
Total Split (s)											
Total Split (%)											
Maximum Green (s)											
Yellow Time (s)											
All-Red Time (s)											
Lost Time Adjust (s)											
Total Lost Time (s)											
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)											
Recall Mode											
Walk Time (s)											
Flash Dont Walk (s)											
Pedestrian Calls (#/hr)											
Act Effect Green (s)											
Actuated q/C Ratio											
Vc Ratio											
Control Delay											
Queue Delay											
Queue Length 50th (m)											
Queue Length 95th (m)											
Internal Link Dist (m)											
Turn Bay Length (m)											
Base Capacity (vph)											
Starvation Cap Reductn											
Spillback Cap Reductn											
Storage Cap Reductn											
Reduced v/c Ratio											
Intersection Summary											

Lanes, Volumes, Timings  
2: Unwin Road & University Drive

Existing Conditions  
AM Peak Hour

Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	173	1	40	18	3	2	117	825	9	6	336	273
Ideal Flow (vphpl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	4.8	4.8	4.8	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Storage Length (m)	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	70.0	0.0	0.0	0.0
Storage Lanes	0	0	0	0	0	1	0	0	1	0	0	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95	0.95
Ped Bike Factor	0.98						0.95	0.99	1.00	0.99	0.98	
Frt	0.975						0.989	0.998			0.933	
Filt Protected	0.961						0.962	0.950			0.950	
Sad. Flow (prot)	0	1804	0	0	1704	0	1640	3399	0	1704	3031	0
Filt Permitted	0.751						0.721	0.400			0.274	
Sad. Flow (perm)	0	1404	0	0	1221	0	681	3399	0	489	3031	0
Right Turn on Red	Yes				Yes		Yes	Yes		Yes		Yes
Sad. Flow (RTOR)	18				2		2			290		
Link Speed (v/h)	50				50		50			50		
Link Distance (m)	304.8				51.0		199.2			212.1		
Travel Time (s)	21.9				3.7		14.3			15.3		
Conf. Ped. (#/hr)	6	89	89	6	20		14	14		20		
Conf. Bikes (#/hr)				1								
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	5%	2%	2%	2%	2%	2%	6%	2%	2%	2%	4%	5%
Bus Blockages (#/hr)	0	6	6	0	0	0	0	11	0	0	11	
Adj. Flow (vph)	184	1	43	19	3	2	124	878	10	6	357	290
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	228	0	0	24	0	124	888	0	6	647	0
Enter Blocked Intersection	No	No	No	No	No							
Lane Alignment	Left	Left	Right	Left	Right	Left	Left	Right	Left	Left	Right	
Median Width(m)	0.0				0.0		3.5			3.5		
Link Offset(m)	0.0				0.0		0.0			0.0		
Crosswalk Width(m)	4.8				4.8		4.8			4.8		
Two way Left Turn Lane												
Headway Factor	0.88	0.91	0.88	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	1		1	1		1	1		1	1	
Detector Template	Left	Thru	Left	Thru	Left	Thru	Left	Thru	Left	Thru		
Leading Detector (m)	2.0	4.0	2.0	4.0	8.0	4.0	8.0	4.0				
Trailing Detector (m)	0.0	2.0	0.0	2.0	2.0	2.0	2.0	2.0				
Detector 1 Position(m)	0.0	2.0	0.0	2.0	2.0	2.0	2.0	2.0				
Detector 1 Size(m)	2.0	2.0	2.0	2.0	6.0	2.0	6.0	2.0				
Detector 1 Type	Cl+Ex											
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Turn Type	Perm		Perm		Perm		Perm		Perm			
Protected Phases	4		8		2		2		6			
Permitted Phases	4		8		2		2		6			

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Lanes, Volumes, Timings  
2: Unwin Road & University Drive

Existing Conditions  
AM Peak Hour

Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	8	8	2	2	6	6	6	6	6	6
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		21.0	21.0		21.0	21.0	
Minimum Split (s)	16.0	16.0		16.0	16.0		26.5	26.5		26.5	26.5	
Total Split (s)	31.0	31.0	0.0	31.0	31.0	0.0	40.5	40.5	0.0	40.5	40.5	0.0
Total Split (%)	43.4%	43.4%	0.0%	43.4%	43.4%	0.0%	56.6%	56.6%	0.0%	56.6%	56.6%	0.0%
Maximum Green (s)	25.0	25.0		25.0	25.0		35.0	35.0		35.0	35.0	
Yellow Time (s)	3.5	3.5		3.5	3.5		4.0	4.0		4.0	4.0	
All-Red Time (s)	2.5	2.5		2.5	2.5		1.5	1.5		1.5	1.5	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.0	6.0	4.0	6.0	6.0	4.0	5.5	5.5	4.0	5.5	5.5	4.0
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		Min	Min		Min	Min	
Walk Time (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0	8.0	
Flash Don't Walk (s)	15.0	15.0		15.0	15.0		13.0	13.0		13.0	13.0	
Pedestrian Calls (#/hr)	20	20		14	14		89	89		6	6	
Act Effct Green (s)	13.4			13.4			22.7	22.7		22.7	22.7	
Actuated g/C Ratio	0.28			0.28			0.47	0.47		0.47	0.47	
v/C Ratio	0.56			0.07			0.39	0.35		0.03	0.41	
Control Delay	19.4			12.6			13.9	11.2		8.8	5.7	
Queue Delay	0.0			0.0			0.0	0.0		0.0	0.0	
Total Delay	19.4			12.6			13.9	11.2		8.8	5.7	
LOS	B			B			B	B		A	A	
Approach Delay	19.4			12.6			11.5			5.7		
Approach LOS	B			B			B			A		
Queue Length 50th (m)	14.6			1.3			5.8	23.8		0.2	8.1	
Queue Length 95th (m)	36.6			6.1			22.9	56.5		2.2	24.0	
Internal Link Dist (m)	280.8			27.0			27.0			175.2		188.1
Turn Bay Length (m)										70.0		70.0
Base Capacity (vph)	760			654			510	2547		366		2343
Starvation Cap Reductn	0			0			0	0		0		0
Spillback Cap Reductn	0			0			0	0		0		0
Storage Cap Reductn	0			0			0	0		0		0
Reduced v/C Ratio	0.30			0.04			0.24	0.35		0.02	0.08	

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HCM Unsignalized Intersection Capacity Analysis

Existing Conditions

PM Peak Hour

Movement	EBT	EBC	WBL	WBT	NBL	NBR
Lane Configurations						
Volume (veh/h)	172	3	40	180	3	80
Sign Control	Free		Free	Stop		
Grade	0%		0%	0%		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	181	3	42	189	3	84
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None					
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume						
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol						
vC, single (s)						
vC, 2 stage (s)						
F (s)						
p0 queue free %						
cM capacity (veh/h)						
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	184	232	87			
Volume Left	0	42	3			
Volume Right	3	0	84			
cSH	1700	1157	747			
Volume to Capacity	0.11	0.04	0.12			
Queue Length 95th (m)	0.0	0.9	3.2			
Control Delay (s)	0.0	1.8	10.5			
Lane LOS	A	B				
Approach Delay (s)	0.0	1.8	10.5			
Approach LOS			B			

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### HCM Unsignalized Intersection Capacity Analysis 1: Unwin Road & Uxbridge Drive

Movement	Existing Conditions PM Peak Hour											
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Sign Control	Stop			Stop			Stop			Stop		
Volume (vph)	0	8	30	236	3	65	9	155	206	69	183	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	0	8	32	248	3	68	9	163	217	73	193	0
<b>Direction, Lane #</b>												
EB 1	WB 1	NB 1	SB 1									
Volume Total (vph)	40	320	389	265								
Volume Left (vph)	0	248	9	73								
Volume Right (vph)	32	68	217	0								
Had (s)	0.44	0.11	0.29	0.15								
Departure Headway (s)	6.0	5.8	5.2	5.8								
Degree Utilization, x	0.07	0.52	0.56	0.43								
Capacity (veh/h)	478	574	655	583								
Control Delay (s)	9.4	15.0	14.6	13.0								
Approach Delay (s)	9.4	15.0	14.6	13.0								
Approach LOS	A	C	B	B								
<b>Intersection Summary</b>												
Average Delay				14.1								
HCM Level of Service				B								
Intersection Capacity Utilization	71.3%			ICU Level of Service				C				
Analysis Period (min)	15											

### HCM Unsignalized Intersection Capacity Analysis 14: North Driveway & Uxbridge Drive

Movement	Existing Conditions PM Peak Hour					
	EBL	EBR	NBL	NBT	SBT	SBR
<b>Lane Configurations</b>						
Sign Control	Stop		Stop		Stop	
Volume (veh/h)	45	77	67	325	393	56
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	47	81	71	342	414	59
<b>Pedestrians</b>						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None		None
Median storage veh						
Upstream signal (m)				223		
pX, platoon unblocked						
vC, conflicting volume	926	443	473			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	926	443	473			
IC, single (s)	6.4	6.2	4.1			
IC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	83	87	94			
cM capacity (veh/h)	279	615	1089			
<b>Direction, Lane #</b>						
EB 1	NB 1	SB 1				
Volume Total	128	413	473			
Volume Left	47	71	0			
Volume Right	81	0	59			
cSH	426	1089	1700			
Volume to Capacity	0.30	0.06	0.28			
Queue Length 95th (m)	10.0	1.7	0.0			
Control Delay (s)	17.1	2.0	0.0			
Lane LOS	C	A				
Approach Delay (s)	17.1	2.0	0.0			
Approach LOS	C					
<b>Intersection Summary</b>						
Average Delay				3.0		
Intersection Capacity Utilization	63.5%			ICU Level of Service		B
Analysis Period (min)	15					

### HCM Unsignalized Intersection Capacity Analysis 16: Ulster Road & Uxbridge Drive

Movement	Existing Conditions PM Peak Hour					
	WBL	WBR	NBT	NBR	SBL	SBT
<b>Lane Configurations</b>						
Volume (veh/h)	21	9	383	24	6	464
Sign Control	Stop		Free			
Grade	0%	0%	0%			
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	22	9	403	25	6	488
Pedestrians	26		63		41	
Lane Width (m)	3.5		3.5		4.8	
Walking Speed (m/s)	1.2		1.2		1.2	
Percent Blockage	2		5		5	
Right turn flare (veh)						
Median type			None			
Median storage veh						
Upstream signal (m)			154			
pX, platoon unblocked	1006	281		454		
vC, conflicting volume	1006	281		454		
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1006	281		454		
IC, single (s)	6.8	6.9		4.1		
IC, 2 stage (s)						
tF (s)	3.5	3.3		2.2		
p0 queue free %	90	99		99		
cM capacity (veh/h)	219	669		1079		
<b>Direction, Lane #</b>						
WB 1	NB 1	NB 2	SB 1			
Volume Total	32	269	160	495		
Volume Left	22	0	0	6		
Volume Right	9	0	25	0		
cSH	275	1700	1700	1079		
Volume to Capacity	0.11	0.16	0.09	0.01		
Queue Length 95th (m)	3.1	0.0	0.0	0.1		
Control Delay (s)	19.8	0.0	0.0	0.2		
Lane LOS	C		A			
Approach Delay (s)	19.8	0.0		0.2		
Approach LOS	C					
<b>Intersection Summary</b>						
Average Delay				0.7		
Intersection Capacity Utilization	47.5%			ICU Level of Service		A
Analysis Period (min)	15					

### HCM Unsignalized Intersection Capacity Analysis 12: South Driveway & Uxbridge Drive

Movement	Existing Conditions PM Peak Hour					
	EBL	EBR	NBL	NBT	SBT	SBR
<b>Lane Configurations</b>						
Volume (veh/h)	6	134	132	401	472	13
Sign Control	Stop		Free		Free	
Grade	0%	0%	0%			
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	6	141	139	422	497	14
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type			None			
Median storage veh						
Upstream signal (m)			95			
pX, platoon unblocked	993	255	511			
vC, conflicting volume	993	255	511			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	993	255	511			
IC, single (s)	6.8	6.9	4.1			
IC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	97	81	87			
cM capacity (veh/h)	210	744	1051			
<b>Direction, Lane #</b>						
EB 1	NB 1	NB 2	NB 3	SB 1	SB 2	
Volume Total	147	139	211	211	331	179
Volume Left	6	139	0	0	0	0
Volume Right	141	0	0	0	0	14
cSH	671	1051	1700	1700	1700	1700
Volume to Capacity	0.22	0.13	0.12	0.12	0.19	0.11
Queue Length 95th (m)	6.7	3.6	0.0	0.0	0.0	0.0
Control Delay (s)	11.9	8.9	0.0	0.0	0.0	0.0
Lane LOS	B	A				
Approach Delay (s)	11.9	2.2			0.0	
Approach LOS	B					
<b>Intersection Summary</b>						
Average Delay				2.5		
Intersection Capacity Utilization	40.2%			ICU Level of Service		A
Analysis Period (min)	15					

Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

Existing Conditions PM Peak Hour												
Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Volume (vph)	98	898	101	237	1051	128	413	324	557	141	211	238
Ideal Flow (vphpl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8
Storage Length (m)	85.0	0.0	75.0	0.0	110.0	0.0	50.0	0.0	50.0	0.0	0.0	0.0
Storage Lanes	1	1	1	1	1	1	0	1	0	1	0	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped/Bike Factor	0.99	0.83	0.96	0.96	0.93	0.91	0.96	0.96	0.96	0.89	0.96	0.95
Frt		0.850		0.850		0.905		0.920				
Fit Protected	0.950		0.950		0.950		0.950		0.950			
Sald. Flow (prot)	1704	3342	1638	1609	3408	1689	1704	2781	0	1704	2690	0
Fit Permitted	0.950		0.950		0.210		0.308					
Sald. Flow (perm)	1695	3342	1360	1539	3408	1628	352	2781	0	532	2690	0
Right Turn on Red		Yes										
Sald. Flow (RTOR)		88		120		325		155				
Link Speed (v/h)	70		70		50		50					
Link Distance (m)	328.0		201.4		258.5		95.4					
Travel Time (s)	16.9		10.4		18.6		6.9					
Conf. Peds. (#/hr)	13	85	85	13	104	74	74	74	74	104		
Conf. Bikes (#/hr)						1				3		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	4%	4%	8%	2%	3%	2%	3%	2%	2%	10%	2%
Bus Blockages (#/hr)	0	0	11	0	0	6	0	0	0	0	0	0
Adj. Flow (vph)	103	945	106	249	1106	135	435	341	586	148	222	251
Shared Lane Traffic (%)												
Lane Group Flow (vph)	103	945	106	249	1106	135	435	927	0	148	473	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right	Right								
Median Width(m)	3.5		3.5		3.5		3.5		3.5			
Link Offset(m)	0.0		0.0		0.0		0.0					
Crosswalk Width(m)	4.8		4.8		4.8		4.8		4.8			
Two way Left Turn Lane												
Headway Factor	1.05	1.05	0.93	1.05	1.05	0.91	1.05	1.05	0.88	1.05	1.05	0.88
Turning Speed (k/h)	25	15	25	15	25	15	25	15	25	15	25	15
Number of Detectors	1	1	1	1	1	1	1	1	1	1	1	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Left	Thru		
Leading Detector (m)	8.0	4.0	8.0	4.0	2.0	8.0	4.0	8.0	4.0	8.0	4.0	8.0
Trailing Detector (m)	2.0	2.0	2.0	2.0	2.0	0.0	2.0	2.0	2.0	2.0	2.0	2.0
Detector 1 Position(m)	2.0	2.0	2.0	2.0	2.0	0.0	2.0	2.0	2.0	2.0	2.0	2.0
Detector 1 Size(m)	6.0	2.0	2.0	6.0	2.0	2.0	6.0	2.0	6.0	2.0	6.0	2.0
Detector 1 Type	Cl+Ex											
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Prot		Perm	Prot		Perm	pm+pt			Perm		
Protected Phases	5	2	1	6	3	8	4			4		
Permitted Phases		2		6	8		4					

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Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

Existing Conditions PM Peak Hour												
Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	5	2	2	1	6	6	3	8	4	4	4	4
Switch Phase												
Minimum Initial (s)	7.0	29.0	29.0	7.0	29.0	29.0	7.0	17.0	10.0	10.0	10.0	10.0
Minimum Split (s)	11.5	35.0	35.0	11.5	35.0	35.0	10.0	29.0	29.0	29.0	29.0	29.0
Total Split (s)	14.5	55.0	55.0	34.5	75.0	75.0	56.0	0.0	29.0	29.0	0.0	0.0
Total Split (%)	10.0%	37.8%	37.8%	23.7%	51.5%	51.5%	18.6%	38.5%	0.0%	19.9%	19.9%	0.0%
Maximum Green (s)	10.0	49.0	49.0	30.0	69.0	69.0	24.0	50.0	23.0	23.0	23.0	23.0
Yellow Time (s)	3.5	4.5	4.5	3.5	4.5	4.5	3.0	3.5	3.5	3.5	3.5	3.5
All-Red Time (s)	1.0	1.5	1.5	1.0	1.5	1.5	0.0	2.5	2.5	2.5	2.5	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	6.0	4.5	6.0	6.0	3.0	6.0	4.0	6.0	6.0	4.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None	None	None
Walk Time (s)	10.0	10.0	10.0	10.0	10.0	10.0	8.0	8.0	8.0	8.0	8.0	8.0
Flash Don't Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	15.0	15.0	15.0	15.0	15.0	15.0
Pedestrian Calls (#/hr)	85	85	13	13	74	74	104	104	104	104	104	104
Act Effct Green (s)	10.1	42.1	42.1	24.8	56.8	56.8	53.5	50.5	23.2	23.2	23.2	23.2
Actuated g/C Ratio	0.08	0.31	0.31	0.19	0.42	0.42	0.40	0.38	0.17	0.17	0.17	0.17
V/C Ratio	0.80	0.90	0.22	0.84	0.77	0.18	1.13	0.74	1.61	0.80		
Control Delay	102.4	56.4	10.5	77.2	36.3	5.3	119.2	28.4	352.6	47.5		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	102.4	56.4	10.5	77.2	36.3	5.3	119.2	28.4	352.6	47.5		
LOS	F	E	B	E	D	A	F	C	F	D		
Approach Delay	5.63				40.3				57.4		120.2	
Approach LOS	E								E			
Queue Length 50th (m)	29.7	134.5	3.5	68.8	135.1	2.4	-126.6	83.1	-61.7	47.7		
Queue Length 95th (m)	#68.2	168.2	17.8	#109.6	160.7	14.4	#211.4	120.1	#113.9	#79.9		
Internal Link Dist (m)		304.0									234.5	71.4
Turn Bay Length (m)	85.0										50.0	
Base Capacity (vph)	128	1233	557	364	1771	904	385	1250	92	594		
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/C Ratio	0.80	0.77	0.19	0.68	0.62	0.15	1.13	0.74	1.61	0.80		

Intersection Summary

Area Type: Other  
Cycle Length: 145.5  
Actuated Cycle Length: 134  
Natural Cycle: T10  
Control Type: Actuated-Uncoordinated  
Maximum V/C Ratio: 1.61

Intersection Signal Delay: 60.0  
Intersection Capacity Utilization: 98.7%

ICU Level of Service: F

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

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Existing Conditions PM Peak Hour												
Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Volume (vph)	98	898	101	237	1051	128	413	324	557	141	211	238
Ideal Flow (vphpl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8	3.5	4.8	3.5
Storage Length (m)	85.0	0.0	75.0	0.0	110.0	0.0	50.0	0.0	50.0	0.0	0.0	0.0
Storage Lanes	1	1	1	1	1	1	0	1	0	1	0	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	0.95	0.95	0.95	1.00	0.95	0.95	0.95
Ped/Bike Factor	0.99	0.83	0.96									



Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

Existing Improved  
AM Peak Hour

Splits and Phases: 7: 16 Avenue & Uxbridge Drive



Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

Existing Improved  
PM Peak Hour

Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	→	↓	↑	→	↓	↑	→	↓	↑	→	↓
Volume (vph)	98	898	101	237	1051	128	413	324	557	141	211	238
Ideal Flow (vphp)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8
Storage Length (m)	85.0		0.0	75.0		0.0	110.0		0.0	50.0		0.0
Storage Lanes	1		1	2		1	1		0	1		0
Taper Length (m)	7.5		7.5	7.5		7.5	7.5		7.5	7.5		7.5
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	1.00		0.87	0.95		0.97	0.94	0.93		0.98	0.91	
Frt			0.850			0.850		0.905			0.920	
Flt Protected	0.950			0.950			0.950			0.950		
Satl. Flow (prot)	1704	3342	1638	3122	3408	1689	1704	2846	0	1704	2766	0
Flt Permitted	0.950			0.950			0.261			0.198		
Satl. Flow (perm)	1697	3342	1423	2951	3408	1638	440	2846	0	347	2766	0
Right Turn on Red	Yes											
Satl. Flow (RTOR)			106			122		248		206		
Link Speed (k/h)	70			70			50		50		50	
Link Distance (m)	328.0			201.4			258.5		90.4			
Travel Time (s)	16.9			10.4			18.6		6.5			
Confli. Peds. (#/hr)	13		85	85		13	104		74	74		104
Confli. Bikes (#/hr)									1			3
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	4%	4%	8%	2%	3%	2%	3%	2%	2%	10%	2%
Bus Blockages (#/hr)	0	0	11	0	0	6	0	0	0	0	0	0
Adj. Flow (vph)	103	945	106	249	1106	135	435	927	0	148	473	0
Shared Lane Traffic (%)									No	No	No	No
Lane Group Flow (vph)	103	945	106	249	1106	135	435	927	0	148	473	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Right	Right
Median Width(m)	7.0			7.0			3.5			3.5		
Link Offset(m)	0.0			0.0			0.0			0.0		
Crosswalk Width(m)	4.8			4.8			4.8			4.8		
Two way Left Turn Lane												
Headway Factor	1.05	1.05	0.93	1.05	1.05	0.91	1.05	1.05	0.88	1.05	1.05	0.88
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	1	1	1	1	1	1	1	1	1	1	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Left	Thru		
Leading Detector (m)	8.0	4.0	4.0	8.0	4.0	2.0	8.0	4.0	8.0	4.0		
Trailing Detector (m)	2.0	2.0	2.0	2.0	2.0	0.0	2.0	2.0	2.0	2.0	2.0	
Detector 1 Position(m)	2.0	2.0	2.0	2.0	2.0	0.0	2.0	2.0	2.0	2.0	2.0	
Detector 1 Size(m)	6.0	2.0	2.0	6.0	2.0	2.0	6.0	2.0	6.0	2.0	6.0	2.0
Detector 1 Type	Cl+Ex											
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Turn Type	Prot		Perm	Prot		Perm	pm+pt		pm+pt			
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases				2			6	8		4		

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Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

Existing Improved  
PM Peak Hour

Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	5	2	2	1	6	6	3	8	7	4		
Switch Phase												
Minimum Initial (s)	7.0	29.0	29.0	7.0	29.0	29.0	7.0	17.0	10.0	10.0		
Minimum Split (s)	11.5	35.0	35.0	11.5	35.0	35.0	12.0	29.0	13.0	29.0		
Total Split (s)	14.0	41.0	41.0	14.0	41.0	41.0	24.0	42.0	0.0	13.0	31.0	0.0
Total Split (%)	12.7%	37.3%	37.3%	12.7%	37.3%	37.3%	21.8%	38.2%	0.0%	11.8%	28.2%	0.0%
Maximum Green (s)	9.5	35.0	35.0	9.5	35.0	35.0	21.0	36.0	10.0	25.0		
Yellow Time (s)	3.5	4.5	4.5	3.5	4.5	4.5	3.0	3.5	3.0	3.5		
All-Red Time (s)	1.0	1.5	1.5	1.0	1.5	1.5	0.0	2.5	0.0	2.5		
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	6.0	4.5	6.0	6.0	3.0	6.0	4.0	3.0	6.0	4.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag		
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		
Recall Mode	None	Min	Min	None	Min	Min	None	None	None	None		
Walk Time (s)	10.0	10.0	10.0	10.0	10.0	10.0	8.0					
Flash Don't Walk (s)	13.0	13.0	13.0	13.0	13.0	15.0						
Pedestrian Calls (#/hr)	104	104	74	74	104							
Act Effect Green (s)	9.1	34.7	34.7	9.5	35.1	35.1	47.3	31.2	33.2	20.2		
Actuated g/C Ratio	0.09	0.33	0.33	0.09	0.33	0.33	0.45	0.30	0.32	0.19		
v/c Ratio	0.70	0.86	0.20	0.88	0.97	0.21	0.96	0.99dr	0.62	0.68		
Control Delay	73.1	42.7	6.3	78.7	56.3	7.2	58.4	39.2	31.0	26.7		
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	73.1	42.7	6.3	78.7	56.3	7.2	58.4	39.2	31.0	26.7		
LOS	E	D	A	E	E	A	E	D	C	C		
Approach Delay	42.1			55.6			45.3			27.7		
Approach LOS	D			E			D			C		
Queue Length 50th (m)	22.6	103.5	0.0	28.7	-129.1	2.0	67.5	77.4	18.8	28.0		
Queue Length 95th (m)	#49.7	#143.7	12.5	#53.9	#182.3	16.0	#132.6	106.3	32.1	46.0		
Internal Link Dist (m)	304.0			177.4			234.5			66.4		
Turn Bay Length (m)	85.0			75.0			110.0			50.0		
Base Capacity (vph)	155	1118	546	284	1140	629	452	1142	239	818		
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Reduced v/c Ratio	0.66	0.85	0.19	0.88	0.97	0.21	0.96	0.81	0.62	0.58		
<b>Intersection Summary</b>												
Area Type:												
Cycle Length: 110												
Actuated Cycle Length: 105												
Natural Cycle: 90												
Control Type: Actuated-Uncoordinated												
Minimum v/c Ratio: 0.97												
Intersection Signal Delay: 45.5												
Intersection LOS: D												
Intersection Capacity Utilization: 95.4%												
ICU Level of Service: F												
Analysis Period (min) 15												
* Volume exceeds capacity, queue is theoretically infinite.												
Queue shown is maximum after two cycles.												

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Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

Existing Improved  
PM Peak Hour

# 95th percentile volume exceeds capacity, queue may be longer.


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HCM Unsignalized Intersection Capacity Analysis  
5: Uxbridge Drive & Lane

2039 Background  
AM Peak Hour

Movement	EBT	EBC	WBL	WBT	NBL	NBR
Lane Configurations						
Volume (veh/h)	237	10	140	362	10	60
Sign Control	Free		Stop			
Grade	0%		0%		0%	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	252	11	149	385	11	64
Pedestrians	1		28	65		
Lane Width (m)	4.3		4.3	3.5		
Walking Speed (m/s)	1.2		1.2	1.2		
Percent Blockage	0		3	5		
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume		328		1006		350
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol		328		1006		350
IC, single (s)		4.1		6.4		6.2
IC, 2 stage (s)						
tF (s)		2.2		3.5		3.3
p0 queue free %		87		95		90
cM capacity (veh/h)		1167		221		638
Direction, Lane #						
	EB 1	WB 1	NB 1			
Volume Total	263	534	74			
Volume Left	0	149	11			
Volume Right	11	0	64			
cSH	1700	1167	502			
Volume to Capacity	0.15	0.13	0.15			
Queue Length 95th (m)	0.0	3.5	4.1			
Control Delay (s)	0.0	3.4	13.4			
Lane LOS	A	B				
Approach Delay (s)	0.0	3.4	13.4			
Approach LOS		B				
Intersection Summary						
Average Delay		3.2				
Intersection Capacity Utilization		61.8%	ICU Level of Service	B		
Analysis Period (min)		15				

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HCM Unsignalized Intersection Capacity Analysis  
1: Secondary Driveway & Uxbridge Drive

2039 Background  
AM Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	NBL	NBR	SBL	SBT	SBR
Lane Configurations										
Sign Control	Stop		Stop		Stop		Stop		Stop	
Grade	0%		0%		0%		0%		0%	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	1	5	25	262	8	161	1	340	140	131
Hourly flow rate (vph)	1	5	27	279	9	171	1	362	149	139
Direction, Lane #										
	EB 1	WB 1	NB 1	SB 1						
Volume Total (vph)	33	459	512	316						
Volume Left (vph)	1	279	1	139						
Volume Right (vph)	27	171	149	0						
Head (s)	0.44	0.04	-0.10	0.18						
Departure Headway (s)	7.7	6.5	6.3	6.9						
Degree Utilization, x	0.07	0.83	0.89	0.61						
Capacity (veh/h)	408	535	557	486						
Control Delay (s)	11.2	34.3	41.1	20.2						
Approach Delay (s)	11.2	34.3	41.1	20.2						
Approach LOS	B	D	E	C						
Intersection Summary										
Avg Delay		33.0								
HCM Level of Service		D								
Intersection Capacity Utilization		87.8%	ICU Level of Service	E						
Analysis Period (min)		15								

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HCM Unsignalized Intersection Capacity Analysis  
16: Main Driveway & Uxbridge Drive

2039 Background  
AM Peak Hour

Movement	EBL	EBT	EBC	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations											
Volume (veh/h)	0	0	0	24	0	9	0	472	23	5	448
Sign Control	Stop		Stop			Free					
Grade	0%		0%		0%		0%				
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	0	0	0	26	0	10	0	502	24	5	477
Pedestrians	48		80		43		77				
Lane Width (m)	3.5		3.5		3.5		4.8				
Walking Speed (m/s)	1.2		1.2		1.2		1.2				
Percent Blockage	4		6		3		9				
Right turn flare (veh)											
Median type											
Median storage (veh)											
Upstream signal (m)											
pX, platoon unblocked											
vC, conflicting volume		873	1142	568	1125	1130	420	525			607
vC1, stage 1 conf vol											
vC2, stage 2 conf vol		873	1142	568	1125	1130	420	525			607
vCu, unblocked vol											
IC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1				4.1
IC, 2 stage (s)											
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2				2.2
p0 queue free %	100	100	100	81	100	98	100				99
cM capacity (veh/h)	193	178	433	133	181	497	998				905
Direction, Lane #											
	EB 1	WB 1	NB 1	NB 2	SB 1						
Volume Total	0	35	251	276	482						
Volume Left	0	26	0	5							
Volume Right	0	10	0	24	0						
cSH	1700	166	998	1700	905						
Volume to Capacity	0.00	0.21	0.00	0.16	0.01						
Queue Length 95th (m)	0.0	6.2	0.0	0.0	0.1						
Control Delay (s)	0.0	32.5	0.0	0.0	0.2						
Lane LOS	A	D		A							
Approach Delay (s)	0.0	32.5	0.0	0.0	0.2						
Approach LOS	A	D									
Intersection Summary											
Avg Delay		1.2									
Intersection Capacity Utilization		47.6%	ICU Level of Service	A							
Analysis Period (min)		15									

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HCM Unsignalized Intersection Capacity Analysis  
12: Tim Hortons & Uxbridge Drive

2039 Background  
AM Peak Hour

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (veh/h)	130	120	375	100	120	352
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Hourly flow rate (vph)	138	128	399	106	128	374
Pedestrians	80					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.2					
Percent Blockage	6					
Right turn flare (veh)						
Median type						
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume		975	333			585
vC1, stage 1 conf vol						
vC2, stage 2 conf vol		975	333			585
vCu, unblocked vol						
IC, single (s)	6.8	6.9				4.1
IC, 2 stage (s)						
tF (s)	3.5	3.3				2.2
p0 queue free %	31	79				86
cM capacity (veh/h)	201	620				922
Direction, Lane #						
	WB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	266	266	239	252	250	
Volume Left	138	0	128	0		
Volume Right	128	0	106	0	0	
cSH	297	1700	1700	922	1700	
Volume to Capacity	0.90	0.16	0.14	0.14	0.15	
Queue Length 95th (m)	65.9	0.0	0.0	3.8	0.0	
Control Delay (s)	67.3	0.0	0.0	5.5	0.0	
Lane LOS	F			A		
Approach Delay (s)	67.3	0.0		2.8		
Approach LOS	F					
Intersection Summary						
Avg Delay		15.2				
Intersection Capacity Utilization		53.6%	ICU Level of Service	A		
Analysis Period (min)		15				

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Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

2039 Background												
AM Peak Hour												
Lane Group	EBL	EBT	EBr	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	136	1580	790	370	1325	210	150	129	280	84	314	84
Ideal Flow (vphpl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8	3.5	4.8	3.5
Storage Length (m)	85.0	85.0	75.0	0.0	110.0	0.0	50.0	0.0	50.0	0.0	50.0	0.0
Storage Lanes	1	1	2	0	2	0	1	0	1	0	1	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	0.91	0.97	0.95	0.95	1.00	0.95	0.95
Ped/Bike Factor	0.99	0.82	0.97	0.98	0.82	0.91	0.95	0.95	0.95	0.93	0.95	0.95
Frt		0.850	0.980		0.897		0.968					
Fit Protected	0.950		0.950		0.950		0.950					
Sald. Flow (prot)	1704	4896	1649	3273	4549	0	3211	2733	0	1687	2976	0
Fit Permitted	0.950		0.950		0.950		0.298					
Sald. Flow (perm)	1680	4896	1358	3186	4549	0	2633	2733	0	505	2976	0
Right Turn on Red	Yes		Yes		Yes		Yes				Yes	
Sald. Flow (RTOR)		240	27			298		20				
Link Speed (v/h)	70		70		50		50					
Link Distance (m)	328.0			201.4		258.5		84.2				
Travel Time (s)	16.9			10.4		18.6		6.1				
Conf. Peds. (#/hr)	71	88	88	71	222		58	58		222		
Conf. Bikes (#/hr)		5		5			5			5		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	2%	2%	2%	3%	6%	2%	5%	5%	4%	3%	6%	2%
Bus Blockages (#/hr)	0	0	14	0	0	8	0	0	0	0	0	0
Adj. Flow (vph)	145	1681	840	394	1410	223	160	137	298	89	334	89
Shared Lane Traffic (%)												
Lane Group Flow (vph)	145	1681	840	394	1633	0	160	435	0	89	423	0
Enter Blocked Intersection	No	No										
Lane Alignment	Left	Left	Right	Left	Right	Left	Left	Right	Left	Left	Right	Left
Median Width(m)	7.0		7.0			7.0		7.0				
Link Offset(m)	0.0		0.0			0.0						
Crosswalk Width(m)	4.8		4.8			4.8		4.8				
Two way Left Turn Lane												
Headway Factor	1.05	1.05	0.95	1.05	1.05	0.88	1.05	1.05	0.88	1.05	1.05	0.88
Turning Speed (k/h)	25		15	25	15	25		15	25	15		15
Number of Detectors	1	1	1	1	1		1	1	1	1		
Detector Template	Left	Thru	Right	Left	Thru	Left	Thru	Left	Thru	Left	Thru	Left
Leading Detector (m)	8.0	4.0	4.0	8.0	4.0		8.0	4.0	8.0	4.0		
Trailing Detector (m)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0		
Detector 1 Position(m)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0		
Detector 1 Size(m)	6.0	2.0	2.0	6.0	2.0		6.0	2.0	6.0	2.0		
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
Turn Type	Prot		Perm	Prot		Prot			pm+pt			
Protected Phases	5	2	1	6	3	8	7	4				
Permitted Phases			2			4						

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Synchro 7 - Report  
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Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

2039 Background												
AM Peak Hour												
Lane Group	EBL	EBT	EBr	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	7.0	29.0	29.0	7.0	29.0		10.0	10.0		4.0	10.0	
Ideal Flow (vphpl)	11.5	35.0	35.0	11.5	35.0		15.0	29.0		9.0	29.0	
Lane Width (m)	27.0	73.0	73.0	27.0	73.0	0.0	16.0	30.0	0.0	15.0	30.0	0.0
Storage Length (m)												
Storage Lanes												
Taper Length (m)												
Lane Util. Factor												
Ped/Bike Factor												
Frt												
Fit Protected												
Sald. Flow (prot)												
Fit Permitted												
Sald. Flow (perm)												
Right Turn on Red												
Sald. Flow (RTOR)												
Link Speed (v/h)												
Link Distance (m)												
Travel Time (s)												
Conf. Peds. (#/hr)												
Conf. Bikes (#/hr)												
Peak Hour Factor												
Heavy Vehicles (%)												
Bus Blockages (#/hr)												
Adj. Flow (vph)												
Shared Lane Traffic (%)												
Lane Group Flow (vph)												
Enter Blocked Intersection												
Lane Alignment												
Median Width(m)												
Link Offset(m)												
Crosswalk Width(m)												
Two way Left Turn Lane												
Headway Factor												
Turning Speed (k/h)												
Number of Detectors												
Detector Template												
Leading Detector (m)												
Trailing Detector (m)												
Detector 1 Position(m)												
Detector 1 Size(m)												
Detector 1 Type												
Detector 1 Channel												
Detector 1 Extend (s)												
Detector 1 Queue (s)												
Detector 1 Delay (s)												
Turn Type	Perm			Perm			Perm			Perm		
Protected Phases	4		8			2			6			
Permitted Phases			2			4						

Stadium Shopping Centre  
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Synchro 7 - Report  
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Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Background												
AM Peak Hour												
Lane Group	EBL	EBT	EBr	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	200	16	60	29	11	21	120	1195	14	13	656	300
Ideal Flow (vphpl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	4.8	4.8	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Storage Length (m)	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	70.0	0.0	70.0	0.0
Storage Lanes	0	0	0	0	0	0	1	0	1	0	1	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95
Ped/Bike Factor	0.96	0.97					0.950				0.950	
Frt												
Fit Protected												
Sald. Flow (prot)	0	1785	0	0	1658	0	1640	3399	0	1704	3102	0
Fit Permitted	0.744						0.821			0.139		
Sald. Flow (perm)	0	1369	0	0	1351	0	383	3399	0	248	3102	0
Right Turn on Red	Yes						Yes			Yes		
Sald. Flow (RTOR)	16						22			2		144

Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Background  
AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8		2	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		21.0	21.0		21.0	21.0	
Minimum Split (s)	28.0	28.0		28.0	28.0		26.5	26.5		26.5	26.5	
Total Split (s)	32.0	32.0	0.0	32.0	32.0	0.0	58.0	58.0	0.0	58.0	58.0	0.0
Total Split (%)	35.6%	35.6%	0.0%	35.6%	35.6%	0.0%	64.4%	64.4%	0.0%	64.4%	64.4%	0.0%
Maximum Green (s)	27.0	27.0		27.0	27.0		53.0	53.0		53.0	53.0	
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	4.0	5.0	5.0	4.0	5.0	5.0	4.0	5.0	5.0	4.0
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		Min	Min		Min	Min	
Walk Time (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0	8.0	
Flash Don't Walk (s)	15.0	15.0		15.0	15.0		13.0	13.0		13.0	13.0	
Pedestrian Calls (#/hr)	20	20		14	14		89	89		6	6	
Act Effct Green (s)	17.7			17.7			32.9	32.9		32.9	32.9	
Actuated g/c Ratio	0.29			0.29			0.54	0.54		0.54	0.54	
Vc Ratio	0.72			0.16			0.62	0.71		0.11	0.59	
Control Delay	32.1			15.2			26.9	13.3		10.2	9.9	
Queue Delay	0.0			0.0			0.0	0.0		0.0	0.0	
Total Delay	32.1			15.2			26.9	13.3		10.2	9.9	
LOS	C			B			C	B		B	A	
Approach Delay	32.1			15.2			14.6				9.9	
Approach LOS	C			B			B				A	
Queue Length 50th (m)	26.2			3.3			9.0	51.0		0.7	30.5	
Queue Length 95th (m)	#75.8			15.5			36.0	96.5		4.1	62.1	
Internal Link Dist (m)	280.8			27.0			175.2				188.1	
Turn Bay Length (m)							70.0				70.0	
Base Capacity (vph)	659			653			327	2899		211	2667	
Starvation Cap Reductn	0			0			0	0		0	0	
Spillback Cap Reductn	0			0			0	0		0	0	
Storage Cap Reductn	0			0			0	0		0	0	
Reduced v/c Ratio	0.45			0.10			0.39	0.44		0.07	0.38	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 61.4

Natural Cycle: 65

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.72

Intersection LOS: B

ICU Level of Service E

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Stadium Shopping Centre  
TIA

Synchro 7 - Report  
Page 2

Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Background  
AM Peak Hour



HCM Unsignedized Intersection Capacity Analysis  
5: Uxbridge Drive & Lane

2039 Background  
PM Peak Hour

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Volume (veh/h)	252	3	40	258	3	80
Sign Control	Free		Free	Stop		
Grade	0%		0%	0%		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	265	3	42	272	3	84
Pedestrians						
Lane Width (m)					4.3	3.5
Walking Speed (m/s)					1.2	1.2
Percent Blockage					4	2
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (m)						
px, platoon unblocked						
VC, conflicting volume					297	652
vC1, stage 1 conf vol						334
vC2, stage 2 conf vol						
vCu, unblocked vol					297	652
tc, single (s)					4.5	6.4
tc, 2 stage (s)						6.2
tf (s)					2.5	3.5
p0 queue free %					96	99
em capacity (veh/h)					1063	406
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	268	314	87			
Volume Left	0	42	3			
Volume Right	3	0	84			
CSH	1700	1063	650			
Volume to Capacity	0.16	0.04	0.13			
Queue Length 95th (m)	0.0	1.0	3.7			
Control Delay (s)	0.0	1.5	11.4			
Lane LOS	A	B				
Approach Delay (s)	0.0	1.5	11.4			
Approach LOS		B				
Intersection Summary						
Average Delay			2.2			
Intersection Capacity Utilization		52.0%		ICU Level of Service		A
Analysis Period (min)		15				

Stadium Shopping Centre  
TIA

Synchro 7 - Report  
Page 1

HCM Unsignedized Intersection Capacity Analysis  
1: Secondary Driveway & Uxbridge Drive

2039 Background  
PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Sign Control	Stop		Stop		Stop		Stop		Stop		Stop	
Volume (vph)	0	8	29	264	3	84	5	214	206	202	130	0
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	0	8	31	278	3	88	5	225	217	213	137	0
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	39	369	447	349								
Volume Left (vph)	0	278	5	213								
Volume Right (vph)	31	88	217	0								
Hadj (s)	-0.44	0.09	-0.25	0.19								
Departure Headway (s)	6.9	6.4	5.8	6.3								
Degree Utilization, x	0.07	0.66	0.72	0.61								
Capacity (veh/h)	399	369	600	533								
Control Delay (s)	10.5	20.8	21.9	18.9								
Approach Delay (s)	10.5	20.8	21.9	18.9								
Approach LOS	B	C	C	C								
Intersection Summary												
Delay								20.3				
HCM Level of Service								C				
Intersection Capacity Utilization								81.9%	ICU Level of Service		D	
Analysis Period (min)								15				

Stadium Shopping Centre  
TIA

Synchro 7 - Report  
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### HCM Unsignalized Intersection Capacity Analysis 16: Main Driveway & Uxbridge Drive

2039 Background  
PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓	↑	↓	↑	↓	↑	↓	↑	↓	↑	↓
Volume (veh/h)	0	0	0	21	0	9	0	416	24	6	417	0
Sign Control	Stop	Stop	Stop	Free								
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	0	0	0	22	0	9	0	438	25	6	439	0
Pedestrians	22				31		76		49			
Lane Width (m)	3.5				3.5		3.5		4.8			
Walking Speed (m/s)	1.2				1.2		1.2		1.2			
Percent Blockage	2				3		6		5			
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (m)									148			
px, platoon unblocked												
vC, conflicting volume	751	968	537	1009	955	312	461		494			
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	751	968	537	1009	955	312	461		494			
IC, single (s)	7.5	6.5	6.9	7.5	6.5	6.9	4.1		4.1			
IC, 2 stage (s)												
If (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2		2.2			
p0 queue free %	100	100	100	87	100	98	100		99			
CM capacity (veh/h)	264	240	450	171	244	631	1077		1039			
Direction, Lane #	EB1	WB1	NB1	NB2	SB1							
Volume Total	0	32	219	244	445							
Volume Left	0	22	0	0	6							
Volume Right	0	9	0	25	0							
cSH	1700	219	1077	1700	1039							
Volume to Capacity	0.00	0.14	0.00	0.14	0.01							
Queue Length 95th (m)	0.0	4.0	0.0	0.0	0.1							
Control Delay (s)	0.0	24.2	0.0	0.0	0.2							
Lane LOS	A	C		A								
Approach Delay (s)	0.0	24.2	0.0	0.0	0.2							
Approach LOS	A	C		A								
<b>Intersection Summary</b>												
Average Delay			0.9									
Intersection Capacity Utilization	46.7%		ICU Level of Service		A							
Analysis Period (min)	15											

### HCM Unsignalized Intersection Capacity Analysis 13: Tim Hortons & Uxbridge Drive

2039 Background  
PM Peak Hour

Movement	WBL	WBR	NBL	NBT	NBR	SBL	SBT					
Lane Configurations	↑	↓	↑	↓	↑	↓	↑					
Volume (veh/h)	10	40	400	60	30	408	0					
Sign Control	Stop	Free	Free	Free	Free	Free	Free					
Grade	0%	0%	0%	0%	0%	0%	0%					
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95					
Hourly flow rate (vph)	11	42	421	63	32	429	0					
Pedestrians	31											
Lane Width (m)	3.5											
Walking Speed (m/s)	1.2											
Percent Blockage	3											
Right turn flare (veh)												
Median type												
Median storage (veh)												
Upstream signal (m)			83									
px, platoon unblocked												
vC, conflicting volume	762	273		515								
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	762	273		515								
IC, single (s)	6.8	6.9		4.1								
IC, 2 stage (s)												
If (s)	3.5	3.3		2.2								
p0 queue free %	97	94		97								
CM capacity (veh/h)	322	706		1020								
Direction, Lane #	WB1	NB1	NB2	SB1	SB2							
Volume Total	53	281	204	175	286							
Volume Left	11	0	0	32	0							
Volume Right	42	0	63	0	0							
cSH	570	1700	1700	1020	1700							
Volume to Capacity	0.09	0.17	0.12	0.03	0.17							
Queue Length 95th (m)	2.4	0.0	0.0	0.8	0.0							
Control Delay (s)	12.0	0.0	0.0	1.8	0.0							
Approach Delay (s)	12.0	0.0	0.0	0.7								
Approach LOS	B		A									
<b>Intersection Summary</b>												
Average Delay			0.9									
Intersection Capacity Utilization	39.5%		ICU Level of Service		A							
Analysis Period (min)	15											

### Lanes, Volumes, Timings 7: 16 Avenue & Uxbridge Drive

2039 Background  
PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓	↑	↓	↑	↓	↑	↓	↑	↓	↑	↓
Volume (vph)	184	1270	90	270	1940	93	410	182	560	154	134	129
Ideal Flow (vphft)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8	3.5	4.8	3.5
Storage Length (m)	85.0	85.0	75.0	0.0	110.0	0.0	50.0	0.0	50.0	0.0	50.0	0.0
Storage Lanes	1	1	2	0	2	1	1	1	0			
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	0.91	0.97	1.00	1.00	1.00	0.95	0.95
Ped/Bike Factor	1.00		0.81	0.95	1.00		0.85		0.83	0.88	0.89	
Ft		0.850		0.993			0.850		0.926			
Ft Protected	0.950		0.950		0.950		0.950		0.950			
Sd ft. Flow (prot)	1704	4802	1638	3122	4851	0	3306	1776	1747	1704	2689	0
Ft Permitted	0.950		0.950		0.950		0.950		0.637			
Sd ft. Flow (perm)	1701	4802	1326	2976	4851	0	2810	1776	1452	1005	2689	0
Right Turn on Red	Yes											
Sd ft. Flow (RTOR)	90		7				354		136			
Link Speed (vph)	70		70				50		50			
Link Distance (m)	328.0		201.4				258.5		82.8			
Travel Time (s)	16.9		10.4				18.6		6.0			
Conf. Peds (#/hr)	16		102	102		16	125		89	89		125
Conf. Bikes (#/hr)		5		5			5		5			
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	4%	8%	2%	3%	2%	3%	2%	2%	10%	2%	
Bus Blockages (#/hr)	0	0	11	0	0	6	0	0	0	0	0	0
Adj. Flow (vph)	194	1337	95	284	2042	98	432	192	589	162	141	136
Shared Lane Traffic (%)												
Lane Group Flow (vph)	194	1337	95	284	2140	0	432	192	589	162	277	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(m)	7.0		7.0				7.0					7.0
Link Offset(m)	0.0		0.0				0.0					
Crosswalk Length(m)	4.8		4.8				4.8					
Two Way Left Turn Lane												
Headway Factor	1.05	1.05	0.93	1.05	1.05	0.88	1.05	1.05	0.88	1.05	1.05	0.88
Turning Speed (vph)	25		15	25		15	25		15	25		15
Number of Detectors	1	1	1	1	1		1	1	1	1		
Detector Template	Left	Thru	Right	Left	Thru		Left	Thru	Right	Left	Thru	
Leading Detector (m)	8.0	4.0	8.0	4.0	8.0		8.0	4.0	2.0	8.0	4.0	
Trailing Detector (m)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	0.0	2.0	2.0	
Detector 1 Position(m)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	0.0	2.0	2.0	
Detector 1 Size(m)	6.0	2.0	2.0	6.0	2.0		6.0	2.0	2.0	6.0	2.0	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extent (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Turn Type	Prot	Perm	Prot	Prot	Perm		Perm	Perm	pm-pt			
Protected Phases	5	2		1	6		3	8	7	4		
Permitted Phases								8	4			

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Lane Group	5	2	2	1	6	3	8	8	8	7	4



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Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

Splits and Phases: 7: 16 Avenue & Uxbridge Drive



2039 Background  
PM Peak Hour

Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Background  
PM Peak Hour

Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	320	16	80	19	11	21	110	845	24	13	1056	231
Ideal Flow (vphp)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	4.8	4.8	4.8	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Storage Length (m)	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	70.0	0.0	0.0	0.0
Storage Lanes	0	0	0	0	0	0	1	0	1	0	0	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	0.92	0.92	0.92	0.95	0.95	0.95	1.00	1.00	1.00	1.00	0.99	0.99
Frt	0.974											
Flt Protected	0.963											
Flt Protected	0	1870	0	0	1584	0	1704	3392	0	1704	3259	0
Flt Permitted	0.740											
Flt Permitted	0	1328	0	0	1364	0	217	3392	0	559	3259	0
Right Turn on Red												
Satl. Flow (RTOR)	17							22		6		43
Link Speed (k/h)	50							50		50		50
Link Distance (m)	304.8							51.0		199.2		212.1
Travel Time (s)	21.9							3.7		14.3		15.3
Conf. Peds. (#/hr)	108							19		10		10
Conf. Bikes (#/hr)								5		5		5
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	3%	3%	3%
Bus Blockages (#/hr)	0	6	6	0	0	0	0	0	9	0	0	9
Adj. Flow (vph)	337	17	84	20	12	22	116	889	25	14	1112	243
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	438	0	0	54	0	116	914	0	14	1355	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Right	Right
Median Width(m)	0.0							0.0				3.5
Link Offset(m)	0.0							0.0				0.0
Crosswalk Width(m)	4.8							4.8				4.8
Two way Left Turn Lane												
Headway Factor	0.88	0.91	0.88	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Turning Speed (k/h)	25			15	25		15	25		15	25	15
Number of Detectors	1	1			1	1		1	1			
Detector Template	Left	Thru										
Leading Detector (m)	2.0	4.0		2.0	4.0		8.0	4.0		8.0	4.0	
Trailing Detector (m)	0.0	2.0		0.0	2.0		2.0	2.0		2.0	2.0	
Detector 1 Position(m)	0.0	2.0		0.0	2.0		2.0	2.0		2.0	2.0	
Detector 1 Size(m)	2.0	2.0		2.0	2.0		6.0	2.0		6.0	2.0	
Detector 1 Type	Cl+Ex	Cl+Ex										
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Turn Type	Perm			Perm			pm+pt			Perm		
Protected Phases	4			8			5			2		6
Permitted Phases	4			8			2			2		6

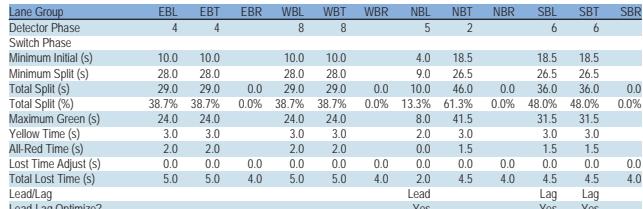
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Lanes, Volumes, Timings  
2: Unwin Road & University Drive

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.



2039 Background  
PM Peak Hour

Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Background  
PM Peak Hour

Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase												
Detector Phase	4	4		8	8		5	2		6	6	
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		4.0	18.5		18.5		
Minimum Split (s)	28.0	28.0		28.0	28.0		9.0	26.5		26.5		
Total Split (s)	29.0	29.0	0.0	29.0	29.0	0.0	10.0	46.0	0.0	36.0	0.0	
Total Split (%)	38.7%	38.7%	0.0%	38.7%	38.7%	0.0%	13.3%	61.3%	0.0%	48.0%	0.0%	
Maximum Green (s)	24.0	24.0		24.0	24.0		8.0	41.5		31.5		
Yellow Time (s)	3.0	3.0		3.0	3.0		2.0	3.0		3.0		
All-Red Time (s)	2.0	2.0		2.0	2.0		0.0	1.5		1.5		
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.0	5.0	4.0	5.0	5.0	4.0	2.0	4.5	4.0	4.5	4.0	
Lead/Lag												
Lead-Lag Optimize?	Yes			Yes			Yes					
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0				
Recall Mode	None	None		None	None		Min	Min				
Walk Time (s)	8.0	8.0		8.0	8.0		8.0	8.0				
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		13.0	13.0				
Pedestrian Calls (#/hr)	8	8		2	2		90	16				
Act Effct Green (s)	24.1			41.2	38.7		31.1	31.1				
Actuated g/C Ratio	0.33			0.33	0.57	0.53	0.43	0.43				
V/C Ratio	0.96			0.12	0.42	0.50	0.06	0.95				
Control Delay	61.6			13.1	11.7	11.6	13.9	36.4				
Queue Delay	0.0			0.0	0.0	0.0	0.0	0.0				
Total Delay	61.6			13.1	11.7	11.6	13.9	36.4				
LOS	E			B			B	D				
Approach Delay	61.6			13.1			11.6					
Approach LOS	E			B			B	D				
Queue Length 50th (m)	-62.9			3.3	6.9	39.8	1.2	98.0				
Queue Length 95th (m)	#123.0			11.0	13.7	54.4	4.7	#148.5				
Internal Link Dist (m)	280.8			27.0			175.2			188.1		
Turn Bay Length (m)							70.0			70.0		
Base Capacity (vph)	454			469	289	1957	245	1449				
Starvation Cap Reductn	0			0	0	0	0	0				
Spillback Cap Reductn	0			0	0	0	0	0				
Storage Cap Reductn	0			0	0	0	0	0				
Reduced v/c Ratio	0.96			0.12	0.40	0.47	0.06	0.94				

Intersection Summary  
Area Type: Other  
Cycle Length: 75  
Actuated Cycle Length: 72.4  
Natural Cycle: 90  
Control Type: Actuated-Uncoordinated  
Maximum v/c Ratio: 0.96  
Intersection Signal Delay: 30.9  
Intersection LOS: C  
Intersection Capacity Utilization: 86.3%  
Analysis Period (min) 15  
Queue exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.

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HCM Unsigned Intersection Capacity Analysis  
5: Uxbridge Drive & Lane

2039 Post Development  
AM Peak Hour

Movement	EBT	EVR	WBL	WTB	NBL	NBR
<b>Lane Configurations</b>						
Volume (veh/h)	255	10	140	367	10	60
Sign Control	Free		Stop			
Grade	0%		0%	0%		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	
Hourly flow rate (vph)	268	11	147	386	11	63
Pedestrians	31		53	84		
Lane Width (m)	4.3		4.3	3.5		
Walking Speed (m/s)	1.2		1.2	1.2		
Percent Blockage	3		5	7		
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (m)		148				
pX, platoon unblocked			0.92			
vC, conflicting volume	363		1070	411		
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	363		1030	411		
IC, single (s)	4.5		6.4	6.2		
IC, 2 stage (s)						
ff (s)	2.5		3.5	3.3		
p0 queue free %	85		94	89		
CM capacity (veh/h)	956		181	566		
<b>Direction, Lane #</b>	<b>EB1</b>	<b>WB1</b>	<b>NB1</b>			
Volume Total	279	534	74			
Volume Left	0	147	11			
Volume Right	11	0	63			
CSH	1700	956	434			
Volume to Capacity	0.16	0.15	0.17			
Queue Length 95th (m)	0.0	4.3	4.8			
Control Delay (s)	0.0	4.0	15.0			
Approach Delay (s)	0.0	4.0	15.0			
Approach LOS			B			
<b>Intersection Summary</b>						
Average Delay		3.6				
Intersection Capacity Utilization	64.8%		ICU Level of Service	C		
Analysis Period (min)	15					

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Lanes, Volumes, Timings

1: Secondary Driveway & Uxbridge Drive

2039 Post Development

AM Peak Hour

Lane Group	EBL	EVR	WBL	WTB	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>										
Volume (vph)	4	15	69	302	49	161	111	342	144	131
Ideal Flow (vphpl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	3.5	3.5	3.5	4.8	4.8	4.8	4.8	4.8	4.8	4.8
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.89					0.89			0.95	0.98
Frt						0.894			0.958	0.967
Flt Protected						0.998			0.971	0.991
Satd. Flow (prot)	0	1432	0	0	1764	0	0	0	1908	0
Flt Permitted						0.984			0.766	0.886
Satd. Flow (perm)	0	1409	0	0	1313	0	0	0	1668	0
Right Turn on Red						Yes			Yes	Yes
Satd. Flow (RTOR)						73			44	39
Link Speed (km/h)	50					50			50	50
Link Distance (m)	99.8					304.8			122.1	148.0
Travel Time (s)	7.2					21.9			8.8	10.7
Conf. Peds. (#/hr)	103					78	78	103	149	70
Conf. Bikes (#/hr)						5	5		5	5
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	2%	2%	5%	2%	5%	2%	2%	3%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	6
Adj. Flow (vph)	4	16	73	318	52	169	117	360	152	138
Shared Lane Traffic (%)										184
Lane Group Flow (vph)	0	93	0	0	539	0	0	629	0	0
Enter Blocked Intersection	No									
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left
Median Width(m)	0.0				0.0			0.0		
Link Offset(m)	0.0				0.0			0.0		
Crosswalk Width(m)	4.8				4.8			4.8		
Two way Left Turn Lane										
Headway Factor	1.05	1.05	1.05	0.88	0.88	0.88	0.88	0.88	0.88	0.91
Turning Speed (km/h)	25		15	25		15	25	15	25	15
Number of Detectors	1	1		1	1		1	1	1	1
Detector Template	Left			Left			Left			Left
Leading Detector (m)	2.0	4.0		2.0	4.0		2.0	4.0		2.0
Trailing Detector (m)	0.0	2.0		0.0	2.0		0.0	2.0		2.0
Detector 1 Position(m)	0.0	2.0		0.0	2.0		0.0	2.0		2.0
Detector 1 Size(m)	2.0	2.0		2.0	2.0		2.0	2.0		2.0
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex
Detector 1 Channel										
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0
Turn Type	Perm			Perm			Perm			Perm
Protected Phases				4			8		2	6
Permitted Phases				4			8		2	6
Detector Phase	4	4			8	8		2	2	6
Switch Phase										
Minimum Initial (s)	4.0	4.0			4.0	4.0		4.0	4.0	4.0

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Lanes, Volumes, Timings										
1: Secondary Driveway & Uxbridge Drive										
2039 Post Development										
AM Peak Hour										
<b>Lane Group</b>										
Minimum Split (s)	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5
Total Split (s)	26.0	26.0	0.0	26.0	0.0	34.0	34.0	0.0	34.0	34.0
Total Split (%)	43.3%	43.3%	0.0%	43.3%	43.3%	0.0%	56.7%	56.7%	0.0%	56.7%
Maximum Green (s)	22.5	22.5	22.5	22.5	30.5	30.5	30.5	30.5	30.5	30.5
Yellow Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	3.5	3.5	4.0	3.5	4.0	3.5	3.5	4.0	3.5	4.0
Lead-Lag										
Lead-Lag Optimize?										
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	Min	Min	Min	Min	Min	Min
Walk Time (s)	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0
Pedestrian Calls (#/hr)	5	5	5	5	5	5	5	5	5	5
Act Effect Green (s)	22.8		22.8			23.2			23.2	
Actuated g/C Ratio	0.43		0.43			0.44			0.44	
v/C Ratio	0.14		0.91			0.84			0.64	
Control Delay	5.6		40.6			23.5			17.7	
Queue Delay	0.0		0.0			0.1			0.0	
Total Delay	5.6		40.6			23.5			17.7	
LOS	A		D			C			B	
Approach Delay	5.6		40.6			23.5			17.7	
Approach LOS	A		D			C			B	
Queue Length 50th (m)	1.1		45.8			48.7			23.8	
Queue Length 95th (m)	9.3		#120.9			86.4			46.3	
Internal Link Dist (m)	75.8		280.8			98.1			124.0	
Turn Bay Length (m)										
Base Capacity (vph)	648		590			989			686	
Starvation Cap Reductn	0		0			20			0	
Spillback Cap Reductn	0		0			0			0	
Storage Cap Reductn	0		0			0			0	
Reduced v/C Ratio	0.14		0.91			0.65			0.48	
<b>Intersection Summary</b>										
Area Type:	Other									
Cycle Length:	60									
Actuated Cycle Length:	53.1									
Natural Cycle:	55									
Control Type:	Actuated-Uncoordinated									
Maximum v/C Ratio:	0.91									
Intersection Signal Delay: 27.1										
Intersection LOS: C										
Intersection Capacity Utilization 82.7%										
ICU Level of Service E										
Analysis Period (min) 15										
# 95th percentile volume exceeds capacity, queue may be longer.										
Queue shown is maximum after two cycles.										

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Lanes, Volumes, Timings  
16: Main Driveway & Uxbridge Drive

2039 Post Development  
AM Peak Hour

Lane Group	EBL	EBT	EPR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	6	0	57	24	0	9	222	582	23	5	492	49
Ideal Flow (vphpl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	4.8	4.8	3.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.86		0.87		0.93	0.99		0.98				
Frt	0.877			0.964		0.994			0.988			
Flt Protected	0.995			0.965		0.950						
Satd. Flow (prot)	0	1358	0	0	1598	0	1704	1753	0	0	1944	0
Flt Permitted	0.969			0.779		0.482			0.997			
Satd. Flow (perm)	0	1307	0	0	1174	0	806	1753	0	0	1938	0
Right Turn on Red				Yes			Yes			Yes		
Satd. Flow (RTOR)	60			9		5				13		
Link Speed (v/h)	50			50		50				50		
Link Distance (m)	50.3			95.6		62.4				122.1		
Travel Time (s)	3.6			6.9		4.5				8.8		
Confl. Peds. (#/hr)	97	156	156	97	134		126	126		134		
Confl. Bikes (#/hr)		5		5		5				5		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	3%	2%	2%	2%	2%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	6	6	6	0	
Adj. Flow (vph)	6	0	60	25	0	9	234	613	24	5	518	52
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	66	0	0	34	0	234	637	0	0	575	0
Enter Blocked Intersection	No	No	No									
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(m)	0.0		0.0		0.0		3.5			3.5		
Link Offset(m)	0.0		0.0		0.0		0.0			0.0		
Crosswalk Width(m)	4.8		4.8		4.8		4.8			4.8		
Two way Left Turn Lane												
Headway Factor	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	0.88	0.91	1.05	
Turning Speed (k/hr)	25		15	25	15	25	15	25	15	25	15	
Number of Detectors	1	1	1	1	1	1	1	1	1	1	1	
Detector Tempalte	Left											
Leading Detector (m)	2.0	4.0	2.0	4.0	8.0	4.0	2.0	4.0				
Trailing Detector (m)	0.0	2.0	0.0	2.0	2.0	2.0	0.0	2.0				
Detector 1 Position(m)	0.0	2.0	0.0	2.0	2.0	2.0	0.0	2.0				
Detector 1 Size(m)	2.0	2.0	2.0	2.0	6.0	2.0	2.0	2.0				
Detector 1 Type	Cl+Ex											
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0				
Turn Type	Perm											
Protected Phases		4			8		2		6			
Permitted Phases	4		8		2		2		6			
Detector Phase	4	4	8	8	2	2	2	2	6	6		
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		

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Lanes, Volumes, Timings  
16: Main Driveway & Uxbridge Drive

2039 Post Development  
AM Peak Hour

Lane Group	EBL	EBT	EPR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Minimum Split (s)	24.5	24.5		24.5	24.5		24.5	24.5		24.5	24.5	
Total Split (s)	25.0	25.0	0.0	25.0	25.0	0.0	35.0	35.0	0.0	35.0	35.0	0.0
Total Split (%)	41.7%	41.7%	0.0%	41.7%	41.7%	0.0%	58.3%	58.3%	0.0%	58.3%	58.3%	0.0%
Maximum Green (s)	21.5	21.5		21.5	21.5		31.5	31.5		31.5	31.5	
Yellow Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
All Red Time (s)	1.5	1.5		1.5	1.5		1.5	1.5		1.5	1.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		Min	Min		Min	Min	
Walk Time (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0	8.0	
Flash Don't Walk (s)	13.0	13.0		13.0	13.0		13.0	13.0		13.0	13.0	
Pedestrian Calls (#/hr)	5	5		5	5		5	5		5	5	
Actuated Effct Green (s)	8.5			8.5			32.7	32.7				
Actuated g/C Ratio	0.21			0.21			0.79	0.79				
v/c Ratio	0.21			0.14			0.37	0.46				
Control Delay	7.6			14.2			7.2	6.2				
Queue Delay	0.0			0.0			0.0	0.0				
Total Delay	7.6			14.2			7.2	6.3				
LOS	A			B			A	A				
Approach Delay	7.6			14.3				6.5				
Approach LOS	A			B				A				
Queue Length 50th (m)	0.4			1.9			5.3	15.9				
Queue Length 95th (m)	7.6			7.3			35.8	81.6				
Internal Link Dist (m)	26.3			71.6			38.4	98.1				
Turn Bay Length (m)												
Base Capacity (vph)	780			682			636	1385				
Starvation Cap Reductn	0			0			0	35				
Spillback Cap Reductn	0			0			0	0				
Storage Cap Reductn	0			0			0	0				
Reduced v/c Ratio	0.08			0.05			0.37	0.47				

Intersection Summary

- Area Type: Other
- Cycle Length: 60
- Actuated Cycle Length: 41.4
- Natural Cycle: 60
- Control Type: Actuated-Uncoordinated
- Maximum v/c Ratio: 0.46
- Intersection Signal Delay: 6.3
- Intersection LOS: A
- Intersection Capacity Utilization: 91.1%
- ICU Level of Service F
- Analysis Period (min) 15

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HCM Unsignalized Intersection Capacity Analysis

12: Tim Hortons & Uxbridge Drive

2039 Post Development

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (veh/h)	130	120	707	100	120	453
Sign Control	Stop		Free		Free	
Grade	0%		0%		0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	137	126	744	105	126	477
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage:						
Right turn flare (veh)						
Median type:						
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1288	425				
vC1, stage 1 conf vol	1288	425				
vC2, stage 2 conf vol	1288	425				
vCu, unblocked vol	1288	425				
vC, single (s)	6.8	6.9				
vC, 2 stage (s)						
F (s)	3.5	3.3				
p0 queue free %	0	78				
cM capacity (veh/h)	130	578				
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	263	496	353	285	318	
Volume Left	137	0	0	126	0	
Volume Right	126	0	105	0	0	
cSH	207	1700	1700	765	1700	
Volume to Capacity	1.27	0.29	0.21	0.17	0.19	
Queue Length 95th (m)	112.4	0.0	0.0	4.7	0.0	
Control Delay (s)	200.4	0.0	0.0	5.8	0.0	
Lane LOS	F			A		
Approach Delay (s)	200.4	0.0		2.7		
Approach LOS	F					
Intersection Summary						
Average Delay				31.7		
Intersection Capacity Utilization				64.7%		
Analysis Period (min)				15		

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## Lanes, Volumes, Timings 7: 16 Avenue & Uxbridge Drive

2039 Post Development  
AM Peak Hour

Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↓	↑↑	→	↓	↑	↓	↑	↓	↑	↓	↑	↓
Volume (vph)	225	1580	790	370	1325	369	150	214	280	145	336	102
Ideal Flow (vphpl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8
Storage Length (m)	85.0	85.0	75.0	0.0	110.0	0.0	50.0	0.0	50.0	0.0	0.0	0.0
Storage Lanes	1	1	1	2	0	2	0	1	0	1	0	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	0.91	0.97	0.95	0.95	1.00	0.95	0.95
Ped/Bike Factor	0.98	0.74	0.96	0.95	0.82	0.88	0.93	0.92	0.92	0.93	0.92	0.95
Frt		0.850	0.967		0.915		0.965		0.965		0.965	
Fit Protected	0.950		0.950		0.950		0.950		0.950		0.950	
Salad. Flow (prot)	1704	4802	1638	3122	4482	0	3306	2720	0	1704	2856	0
Fit Permitted	0.950		0.950		0.950		0.950		0.950		0.950	
Salad. Flow (perm)	1671	4802	1209	2991	4482	0	2725	2720	0	302	2856	0
Right Turn on Red	Yes		Yes		Yes		Yes		Yes		Yes	
Salad. Flow (RTOR)		341		53		195		23				
Link Speed (kph)	70		70		50		50					
Link Distance (m)	328.0			201.4		258.5		85.4				
Travel Time (s)	16.9		10.4		18.6		6.1					
Conf. Peds. (#/hr)	120		134		134		120	246	109	109	246	
Conf. Bikes (#/hr)		5		5		5		5		5		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	4%	4%	8%	2%	3%	2%	3%	2%	2%	10%	2%
Bus Blockages (#/hr)	0	0	11	0	0	6	0	0	0	0	0	0
Adj. Flow (vph)	237	1663	832	389	1395	388	158	225	295	153	354	107
Shared Lane Traffic (%)												
Lane Group Flow (vph)	237	1663	832	389	1783	0	158	520	0	153	461	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Right	Left	Left	Right	Left	Left	Right	
Median Width(m)	7.0		7.0		7.0		7.0		7.0		7.0	
Link Offset(m)	0.0		0.0		0.0		0.0		0.0		0.0	
Crosswalk Width(m)	4.8		4.8		4.8		4.8		4.8		4.8	
Two way Left Turn Lane												
Headway Factor	1.05	1.05	0.93	1.05	1.05	0.88	1.05	1.05	0.88	1.05	1.05	0.88
Turning Speed (kph)	25		15	25	15	25	15	25	15	25	15	
Number of Detectors	1	1	1	1	1		1	1	1	1	1	
Detector Template	Left	Thru	Right	Left	Thru		Left	Thru	Left	Thru		
Leading Detector (m)	8.0	4.0	4.0	8.0	4.0		8.0	4.0	8.0	4.0		
Trailing Detector (m)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0		
Detector 1 Position(m)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0		
Detector 1 Size(m)	6.0	2.0	2.0	6.0	2.0		6.0	2.0	6.0	2.0		
Detector 1 Type	Cli+Ex	Cli+Ex	Cli+Ex	Cli+Ex	Cli+Ex		Cli+Ex	Cli+Ex	Cli+Ex	Cli+Ex		
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
Turn Type	Prot		Perm	Prot		Prot		pm+pt				
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2					4				

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## Lanes, Volumes, Timings 7: 16 Avenue & Uxbridge Drive

2039 Post Development  
AM Peak Hour

Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	5	2	2	1	6			3	8		7	4
Switch Phase												
Minimum Initial (s)	7.0	29.0	29.0	7.0	29.0		7.0	17.0		10.0	10.0	
Minimum Split (s)	11.5	35.0	35.0	11.5	35.0		12.0	29.0		13.0	29.0	
Total Split (s)	34.0	63.0	63.0	27.0	56.0	0.0	25.0	30.0	0.0	25.0	30.0	0.0
Total Split (%)	23.4%	43.4%	43.4%	18.6%	38.6%	0.0%	17.2%	20.7%	0.0%	17.2%	20.7%	0.0%
Maximum Green (s)	29.5	57.0	57.0	22.5	50.0		20.0	24.0		22.0	24.0	
Yellow Time (s)	3.5	4.5	4.5	3.5	4.5		3.0	3.5		3.0	3.5	
All-Red Time (s)	1.0	1.5	1.5	1.0	1.5		2.0	2.5		0.0	2.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	4.5	6.0	6.0	4.5	6.0	4.0	5.0	6.0	4.0	3.0	6.0	4.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	Min	Min	None	Min		None	None		None	None	
Walk Time (s)	10.0	10.0	10.0	10.0	10.0		8.0	8.0		8.0	8.0	
Flash Don't Walk (s)	13.0	13.0	13.0	13.0	13.0		15.0	15.0		15.0	15.0	
Pedestrian Calls (#/hr)	104	104	74		74		104			13		
Act Effct Green (s)	23.2	57.1	57.1	20.6	54.5		11.6	23.7		42.2	24.6	
Actuated g/C Ratio	0.17	0.42	0.42	0.15	0.40		0.09	0.17		0.31	0.18	
V/C Ratio	0.81	0.82	1.18	0.82	0.97		0.56	0.82		0.62	0.86	
Control Delay	75.8	39.7	116.4	70.8	54.2		67.7	45.2		45.6	67.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	75.8	39.7	116.4	70.8	54.2		67.7	45.2		45.6	67.8	
LOS	E	D	F	E	D		E	D		D	E	
Approach Delay	6.62					57.2				50.4		62.3
Approach LOS	E					D				E		
Queue Length 50th (m)	65.3	152.6	-224.3	55.1	175.5		22.6	48.8		32.5	65.0	
Queue Length 95th (m)	97.3	188.0	#320.9	#79.0	#250.8		35.4	#79.8		51.5	#93.6	
Internal Link Dist (m)	304.0					177.4				234.5		61.4
Turn Bay Length (m)	85.0					75.0				110.0		50.0
Base Capacity (vph)	372	2023	707	519	1836		489	643		327	543	
Starvation Cap Reductn	0	0	0	0	0		0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0		0	0		0	0	
Storage Cap Reductn	0	0	0	0	0		0	0		0	0	
Reduced v/c Ratio	0.64	0.82	1.18	0.75	0.97		0.32	0.81		0.47	0.85	

Intersection Summary  
Area Type: Other  
Cycle Length: 145  
Actuated Cycle Length: 135.5  
Natural Cycle: 140  
Control Type: Actuated-Uncoordinated  
Maximum v/c Ratio: 1.18  
Intersection Signal Delay: 60.9  
Intersection Capacity Utilization 100.8%  
ICU Level of Service G  
Analysis Period (min) 15  
- Volume exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.

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## Lanes, Volumes, Timings 2: Unwin Road & University Drive

2039 Post Development  
AM Peak Hour

Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↓	↑	↓	↑	↓	↑	↑↓	↑↑	↓↓	↑↑	↑↓	↑↑
Volume (vph)	212	16	61	29	11	21	151	1196	14	13	659	350
Ideal Flow (vphpl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	4.8	4.8	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Storage Length (m)	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	70.0	0.0	0.0	0.0
Storage Lanes	0	0	0	0	0	0	0	1	0	0	1	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped/Bike Factor	0.94	0.972	0.954		0.998		1.00		0.99	0.96		
Frt	0.972		0.954		0.998		1.00		0.99	0.96		
Fit Protected	0.965		0.977		0.950		0.950					
Salad. Flow (prot)	0	1822	0	0	1640	0	1704	3396	0	1704	3056	0
Fit Permitted	0.742		0.819		0.126					0.208		
Salad. Flow (perm)	0	1361	0	0	1328	0	226	3396	0	368	3056	0
Right Turn on Red	Yes		Yes		Yes		Yes		Yes			
Salad. Flow (RTOR)	16		22				2				137	
Link Speed (kph)	50		50									

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Lanes, Volumes, Timings  
2: Unwin Road & University Drive

Splits and Phases: 2: Unwin Road & University Drive

Phase	Direction	Duration (s)
e2	Eastbound	95
e4	Westbound	34
e6	Westbound	64
e8	Eastbound	34

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HCM Unsignalized Intersection Capacity Analysis							2039 Post Development	
5: Uxbridge Drive & Lane							PM Peak Hour	
Movement	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations	→	↗	↖	↙	↖	↗		
Volume (veh/h)	268	3	40	286	3	80		
Sign Control	Free			Free		Stop		
Grade	0%			0%		0%		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95		
Hourly flow rate (vph)	282	3	42	301	3	84		
Pedestrians	21			53		45		
Lane Width (m)	4.3			4.3		3.5		
Walking Speed (m/s)	1.2			1.2		1.2		
Percent Blockage	2			5		4		
Right turn flare (veh)								
Median type	None			None				
Median storage (veh)								
Upstream signal (m)				148				
pX, platoon unblocked								
vC, conflicting volume								
vC1, stage 1 cont vol				330		735		382
vC2, stage 2 cont vol								
vCu, unblocked vol				330		735		382
IC, single (s)				4.5		6.4		6.2
IC, 2 stage (s)								
IF (s)				2.5		3.5		3.3
p0 queue free %				96		99		86
cm capacity (veh/h)				1018		350		607
Direction, Lane #	EB 1	WB 1	NB 1					
Volume Total	285	343	87					
Volume Left	0	42	3					
Volume Right	3	0	84					
cSH	1700	1018	592					
Volume to Capacity	0.17	0.04	0.15					
Queue Length 95th (m)	0.0	1.0	4.1					
Control Delay (s)	0.0	1.4	12.1					
Lane LOS		A	B					
Approach Delay (s)	0.0	1.4	12.1					
Approach LOS			B					
Intersection Summary								
Average Delay				2.2				
Intersection Capacity Utilization				55.1%		ICU Level of Service		B
Analysis Period (min)				15				

Lanes, Volumes, Timings										2039 Post Development			
1: Secondary Driveway & Uxbridge Drive										PM Peak Hour			
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Volume (vph)	14	74	157	297	38	84	101	228	269	202	135	11	
Ideal Flow (vphpl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	
Lane Width (m)	3.5	3.5	3.5	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	4.8	
Lane Util Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Ped/Bike Factor	0.89				0.91			0.93			0.98		
Frt		0.914				0.973			0.939			0.996	
Flt Protected		0.997				0.966			0.992			0.972	
Flt. Prod. (vph)	0	1456	0	0	1836	0	0	1818	0	0	1890	0	
Flt Permitted		0.972				0.623			0.891			0.467	
Flt. Prod. (perm)	0	1416	0	0	1107	0	0	1600	0	0	895	0	
Right Turn on Red				Yes			Yes			Yes		Yes	
Sal. Flow (RTOR)		165			25			93			4		
Link Speed (kph)		50			50			50			50		
Link Distance (m)		99.8			304.8			120.7			148.0		
Travel Time (s)		7.2			21.9			8.7			10.7		
Conf. Pds. (#hr)	69		105	105		69	165		57	57		165	
Conf. Bikes (#hr)			5			5			5	5		5	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
Heavy Vehicles (%)	2%	2%	2%	5%	2%	5%	2%	2%	3%	2%	7%	2%	
Bus Blockages (#hr)	0	0	0	0	0	0	0	0	0	0	0	0	
Avg. Flow (vph)	15	78	165	313	40	88	106	240	283	213	142	12	
Shared Lane Traffic (%)													
Group Flow (vph)	0	258	0	0	441	0	0	629	0	0	367	0	
Enter Blocked Intersection	No	No	No										
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right	
Median Width(m)	0.0			0.0			0.0			0.0			
Link Offset(m)	0.0			0.0			0.0			0.0			
Crosswalk Width(m)	4.8			4.8			4.8			4.8			
Two way Left Turn Lane													
Headway Factor	1.05	1.05	1.05	0.88	0.88	0.88	0.88	0.88	0.88	0.88	0.91	0.88	
Turning Speed (kph)	25		15	25		15	25		15	25		15	
Number of Detectors	1	1		1	1		1	1		1	1		
Detector Template				Left			Left			Left			
Leading Detector (m)	2.0	4.0		2.0	4.0		2.0	4.0		2.0	4.0		
Trailing Detector (m)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0		
Detector 1 Position(m)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0		
Detector 1 Size(m)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0		
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		
Detector 1 Channel													
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0		
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0		
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0		
Turn Type	Perm			Perm			Perm			Perm			
Protected Phases		4			8			2			6		
Permitted Phases	4			8			2			6			
Detector Phase	4	4		8	8		2	2		6	6		
Switch Phase													
Minimum Initial (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0		

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Lanes, Volumes, Timings 1: Secondary Driveway & Uxbridge Drive											
2039 Post Development PM Peak Hour											
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Split (s)	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5	25.5
Total Split (s)	28.0	28.0	0.0	28.0	28.0	0.0	32.0	32.0	0.0	32.0	0.0
Total Split (%)	46.7%	46.7%	0.0%	46.7%	46.7%	0.0%	53.3%	53.3%	0.0%	53.3%	0.0%
Maximum Green (s)	24.5	24.5	24.5	24.5	24.5	28.5	28.5	28.5	28.5	28.5	28.5
Yellow Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	3.5	4.0
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	Min	Min	Min	Min	Min	Min	Min
Walk Time (s)	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0
Pedestrian Calls (#/hr)	5	5	5	5	5	5	5	5	5	5	5
Act Effct Green (s)	24.3	24.3	24.3	25.1		25.1					
Actuated g/C Ratio	0.43	0.43	0.43	0.45		0.45					
v/c Ratio	0.37	0.37	0.90	0.92		0.92					
Control Delay	6.6	6.6	41.7	22.6		46.5					
Queue Delay	0.0	0.0	0.0	0.2		0.0					
Total Delay	6.6	6.6	41.7	22.8		46.5					
LOS	A	D	C	D							
Approach Delay	6.6	6.6	41.7	22.8		46.5					
Approach LOS	A	D	C	D							
Queue Length 50th (m)	6.5		44.8	48.1		34.8					
Queue Length 95th (m)	20.3		#100.3	#105.9		#84.6					
Internal Link Dist (m)	75.8		280.8	96.7		124.0					
Turn Bay Length (m)											
Base Capacity (vph)	715		500	863		459					
Starvation Cap Reductn	0		0	22		0					
Spillback Cap Reductn	0		0	0		0					
Storage Cap Reductn	0		0	0		0					
Reduced v/c Ratio	0.36		0.88	0.75		0.80					
<b>Intersection Summary</b>											
Area Type:	Other										
Cycle Length:	60										
Actuated Cycle Length:	56.4										
Natural Cycle:	55										
Control Type:	Actuated-Uncoordinated										
Maximum v/c Ratio:	0.92										
Intersection Signal Delay:	30.4										
Intersection LOS:	C										
Intersection Capacity Utilization:	106.2%										
Analysis Period (min)	15										
# 95th percentile volume exceeds capacity, queue may be longer.											
Queue shown is maximum after two cycles.											

Lanes, Volumes, Timings 1: Secondary Driveway & Uxbridge Drive											
2039 Post Development PM Peak Hour											
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Split (s)	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5
Total Split (s)	25.0	25.0	0.0	25.0	25.0	0.0	35.0	35.0	0.0	35.0	0.0
Total Split (%)	41.7%	41.7%	0.0%	41.7%	41.7%	0.0%	58.3%	58.3%	0.0%	58.3%	0.0%
Maximum Green (s)	21.5	21.5	21.5	21.5	21.5	31.5	31.5	31.5	31.5	31.5	31.5
Yellow Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	3.5	4.0
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	Min	Min	Min	Min	Min	Min	Min
Walk Time (s)	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0
Pedestrian Calls (#/hr)	5	5	5	5	5	5	5	5	5	5	5
Act Effct Green (s)	14.9		14.9	21.4		21.4					
Actuated g/C Ratio	0.34		0.34	0.49		0.49					
v/c Ratio	0.77		0.07	0.67		0.67					
Control Delay	20.1		10.4	22.4		13.3					
Queue Delay	0.0		0.0	0.0		0.0					
Total Delay	20.1		10.4	22.4		13.3					
LOS	C	B	C	B							
Approach Delay	20.1		10.4	15.8		12.7					
Approach LOS	C	B	B	B							
Queue Length 50th (m)	15.5		1.1	11.2		29.8					
Queue Length 95th (m)	#68.3		6.4	#45.6		69.0					
Internal Link Dist (m)	26.3		71.6			35.4					
Turn Bay Length (m)											
Base Capacity (vph)	782		708	485		1329					
Starvation Cap Reductn	0		0	0		0					
Spillback Cap Reductn	0		0	0		0					
Storage Cap Reductn	0		0	0		0					
Reduced v/c Ratio	0.54		0.04	0.43		0.42					
<b>Intersection Summary</b>											
Area Type:	Other										
Cycle Length:	60										
Actuated Cycle Length:	44.1										
Natural Cycle:	60										
Control Type:	Actuated-Uncoordinated										
Maximum v/c Ratio:	0.77										
Intersection Signal Delay:	15.6										
Intersection LOS:	B										
Intersection Capacity Utilization:	101.8%										
Analysis Period (min)	15										
# 95th percentile volume exceeds capacity, queue may be longer.											
Queue shown is maximum after two cycles.											

Lanes, Volumes, Timings 1: Secondary Driveway & Uxbridge Drive											
2039 Post Development PM Peak Hour											
Lane Group	EBL	EBT	EBR	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Minimum Split (s)	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5	24.5
Total Split (s)	25.0	25.0	0.0	25.0	25.0	0.0	35.0	35.0	0.0	35.0	0.0
Total Split (%)	41.7%	41.7%	0.0%	41.7%	41.7%	0.0%	58.3%	58.3%	0.0%	58.3%	0.0%
Maximum Green (s)	21.5	21.5	21.5	21.5	21.5	31.5	31.5	31.5	31.5	31.5	31.5
Yellow Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
All-Red Time (s)	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5	1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	3.5	3.5	4.0	3.5	3.5	4.0	3.5	3.5	4.0	3.5	4.0
Lead/Lag											
Lead-Lag Optimize?											
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	None	None	None	Min	Min	Min	Min	Min	Min	Min
Walk Time (s)	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0	8.0
Flash Dont Walk (s)	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0	13.0
Pedestrian Calls (#/hr)	5	5	5	5	5	5	5	5	5	5	5
Act Effct Green (s)	14.9		14.9	21.4		21.4					
Actuated g/C Ratio	0.34		0.34	0.49		0.49					
v/c Ratio	0.77		0.07	0.67		0.67					
Control Delay	20.1		10.4	22.4		13.3					
Queue Delay	0.0		0.0	0.0		0.0					
Total Delay	20.1		10.4	22.4		13.3					
LOS	C	B	C	B							

Lanes, Volumes, Timings  
16: Main Driveway & Uxbridge Drive



2039 Post Development  
PM Peak Hour

HCM Unsignalized Intersection Capacity Analysis  
12: Tim Hortons & Uxbridge Drive

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	Y	Y	Y	Y	Y
Volume (veh/h)	10	40	690	60	30	855
Sign Control	Stop	Free			Free	
Grade	0%	0%			0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	11	42	726	63	32	900
Pedestrians	69	69			69	
Lane Width (m)	3.5	3.5			3.5	
Walking Speed (m/s)	1.2	1.2			1.2	
Percent Blockage	6	6			6	
Right turn flare (veh)						
Median type			None		None	
Median storage (veh)						
Upstream signal (m)			88		59	
p <sub>x</sub> , platoon unblocked						
vC, conflicting volume	1409	533		858		
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1409	533		858		
vC, single (s)	6.8	6.9		4.1		
vC, 2 stage (s)						
IF (s)	3.5	3.3		2.2		
p <sub>d</sub> queue free %	90	90		96		
cm capacity (veh/h)	111	438		735		
Direction, Lane #	WB 1	NB 1	NB 2	SB 1	SB 2	
Volume Total	53	484	305	332	600	
Volume Left	11	0	0	32	0	
Volume Right	42	0	63	0	0	
cSH	275	1700	1700	735	1700	
Volume to Capacity	0.19	0.28	0.18	0.04	0.35	
Queue Length 95th (m)	5.5	0.0	0.0	1.1	0.0	
Control Delay (s)	21.2	0.0	0.0	1.4	0.0	
Lane LOS	C		A			
Approach Delay (s)	21.2	0.0	0.0	0.5		
Approach LOS	C					
<b>Intersection Summary</b>						
Average Delay			0.9			
Intersection Capacity Utilization		66.0%		ICU Level of Service		C
Analysis Period (min)		15				

Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

2039 Post Development  
PM Peak Hour

Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Y	Y	Y	Y	Y	Y	Y	Y	Y	Y	Y	Y
Volume (vph)	251	1265	90	270	1932	213	410	286	560	318	260	287
Ideal Flow (vphpl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8	3.5	4.8	3.5
Storage Length (m)	85.0	85.0	75.0	85.0	110.0	0.0	50.0	0.0	50.0	0.0	50.0	0.0
Storage Lanes	1	1	2	0	2	1	1	1	0	0	0	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	0.97	1.00	1.00	1.00	0.95	0.95	0.95
Ped/Bike Factor	1.00	0.77	0.94	0.99	0.88	0.78	0.91	0.86	0.85	0.921		
FIT		0.850	0.985			0.850			0.921			
FIT Protected	0.950		0.950		0.950		0.950		0.950			
Satd. Flow (prot)	1704	4802	1638	3122	4765	0	3306	1776	1747	1704	2599	0
FIT Permitted	0.950		0.950		0.950		0.950		0.950			
Satd. Flow (perm)	1697	4802	1253	2938	4765	0	2921	1776	1367	388	2599	0
Right Turn on Red	Yes											
Satd. Flow (RTOR)	93		16		258		161					
Link Speed (kph)	70		70		50		50					
Link Distance (m)	328.0		201.4		258.5		88.4					
Travel Time (s)	16.9		10.4		18.6		6.4					
Conf. Peds (#/hr)	56		128	128	56	147	117	117	117	147		
Conf. Bikes (#/hr)	5		5	5	5	5	5	5	5	5		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95		
Heavy Vehicles (%)	2%	4%	8%	2%	3%	2%	3%	2%	10%	2%		
Bus Blockages (#/hr)	0	0	11	0	0	6	0	0	0	0		
Adj. Flow (vph)	264	1332	95	284	2034	224	432	301	589	335	274	302
Shared Lane Traffic (%)												
Lane Group Flow (vph)	264	1332	95	284	2258	0	432	301	589	335	576	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Right	Right
Median Width(m)	7.0		7.0		7.0		7.0		7.0		7.0	
Link Offset(m)	0.0		0.0		0.0		0.0		0.0		0.0	
Crosswalk Width(m)	4.8		4.8		4.8		4.8		4.8		4.8	
Two Way Left Turn Lanes												
Headway Factor	1.05	1.05	0.93	1.05	1.05	0.88	1.05	1.05	0.88	1.05	1.05	0.88
Turning Speed (kph)	25		15	25	15	25	15	25	15	25	15	15
Number of Detectors	1	1	1	1	1	1	1	1	1	1	1	1
Detector Template	Left	Thru	Right	Left	Left	Thru	Right	Left	Left	Thru	Right	Thru
Leading Detector (m)	8.0	4.0	8.0	4.0	8.0	4.0	2.0	8.0	4.0	8.0	4.0	4.0
Trailing Detector (m)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0	2.0	2.0	2.0	2.0
Detector 1 Position(m)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0	2.0	2.0	2.0	2.0
Detector 1 Size(m)	6.0	2.0	2.0	6.0	2.0	6.0	2.0	2.0	6.0	2.0	6.0	2.0
Detector 1 Type	Cl+Ex											
Detector 1 Channel												
Detector 1 Extent (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Turn Type	Prot	Perm	Prot	Prot	Perm	Perm	pm-pt					
Protected Phases	5	2	1	6	3	8	7	4				
Permitted Phases		2			8		7	4				

Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

Lane Group	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	5	2	2	1	6	3	8	8	8	7	4	
Switch Phase												
Minimum Initial (s)	7.0	29.0	29.0	7.0	29.0		7.0	17.0	17.0	10.0	10.0	
Minimum Split (s)	11.5	35.0	35.0	11.5	35.0		12.0	29.0	29.0	13.0	29.0	
Total Split (s)	21.0	53.0	53.0	27.0	59.0	0.0	25.0	32.0	32.0	23.0	30.0	0.0
Total Split (%)	15.6%	39.3%	39.3%	20.0%	43.7%	0.0%	18.5%	23.7%	23.7%	17.0%	22.2%	0.0%
Maximum Green (s)	16.5	47.0	47.0	22.5	53.0	20.0	26.0	26.0	20.0	24.0		
Yellow Time (s)	3.5	4.5	4.5	3.5	4.5		3.0	3.5	3.5	3.0	3.5	
All Red Time (s)	1.0	1.5	1.5	1.0	1.5	2.0	2.5	2.5	2.0	2.5	2.0	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	6.0	4.5	6.0	4.0	5.0	6.0	6.0	5.0	6.0	4.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag	Lag	Lag	Lag	Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	Min	Min	None	Min		None	None	None	None	None	
Walk Time (s)	10.0	10.0	10.0	10.0	10.0		8.0	8.0	8.0	8.0	8.0	
Flash Don't Walk (s)	13.0	13.0	13.0	13.0	13.0		15.0	15.0	15.0	15.0	15.0	
Pedestrian Call (#/hr)	104	104	104	74			104	104				13
Act. Effic. Green (s)	16.5	52.1	52.1	17.4	53.0		19.6	26.0	26.0	47.4	24.4	
Actuated g/C Ratio	0.12	0.39	0.39	0.13	0.39	0.15	0.19	0.19	0.35	0.18		
v/C Ratio	1.27	0.72	0.72	0.18	0.70	1.20	0.90	0.88	1.25	1.01	0.93	
Control Delay	200.0	38.5	6.7	65.8	132.4		79.0	79.3	153.6	86.8	60.2	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	200.0	38.5	6.7	65.8	132.4		79.0	79.3	153.6	86.8	60.2	
LOS	F	D	A	E	F		E	E	F	F	E	
Approach Delay												
Approach LOS	E						F					
Queue Length 50th (m)	-93.1	115.4	0.3	39.9	-281.3		62.0	83.0	-144.3	-73.8	60.6	
Queue Length 95th (m)	#150.2	141.9	12.9	53.6	#311.1		#91.0	#135.1	#219.4	#138.4	#98.4	
Internal Link Dist (m)	304.0											234.5
Turn Bay Length (m)	85.0											50.0
Base Capacity (vph)	208	1852	540	520	1880		490	3				

Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

Splits and Phases: 7: 16 Avenue & Uxbridge Drive



2039 Post Development  
PM Peak Hour

Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Post Development  
PM Peak Hour

Stadium Shopping Centre  
TIA

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Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Post Development  
PM Peak Hour

Lane Group											EGL	EGL	EGL	WGL	WGL	WGL	WGL	WGL	WGL	WGL	WGL	NGL	SGL	SGL	SGL	SGL	SGL	SGL											
Detector Phase											4	4		8	8		5	2		6		6																	
Switch Phase																																							
Minimum Initial (s)	10.0	10.0		10.0	10.0			4.0	18.5			18.5			18.5																								
Minimum Split (s)	28.0	28.0		28.0	28.0			9.0	26.5			25.5			25.5																								
Total Split (s)	32.0	32.0	0.0	32.0	32.0		0.0	10.0	43.0	0.0		33.0			33.0		0.0																						
Total Split (%)	42.7%	42.7%		42.7%	42.7%		0.0%	13.3%	57.3%	0.0%		44.0%			44.0%																								
Maximum Green (s)	27.0	27.0	0.0%	27.0	27.0			7.0	38.5			28.5			28.5																								
Yellow Time (s)	3.0	3.0		3.0	3.0			3.0	3.0			3.0			3.0																								
All-Red Time (s)	2.0	2.0		2.0	2.0			0.0	1.5			1.5			1.5																								
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0			0.0	0.0			0.0			0.0																								
Total Lost Time (s)	5.0	5.0	4.0	5.0	5.0			4.0	3.0	4.5	4.0	4.5			4.5																								
Lead/Lag																																							
Lead-Lag Optimize?																																							
Vehicle Extension (s)	3.0	3.0		3.0	3.0			3.0	3.0			3.0			3.0																								
Recall Mode	None	None		None	None			None	Min			Min			Min																								
Walk Time (s)	8.0	8.0		8.0	8.0			8.0	8.0			8.0			8.0																								
Flash Don't Walk (s)	15.0	15.0		15.0	15.0			13.0	13.0			13.0			13.0																								
Pedestrian Calls (#/hr)	8	8		2	2			90	16			16			16																								
Act Effct Green (s)	27.1			27.1				37.9	36.4			28.6			28.6																								
Actuated q/C Ratio	0.37			0.37				0.52	0.50			0.39			0.39																								
Vlc Ratio	1.13			0.11				0.55	0.54			0.07			1.08																								
Control Delay	105.9			11.6				18.3	13.8			16.0			75.0																								
Queue Delay	0.0			0.0				0.0	0.0			0.0			0.0																								
Total Delay	105.9			11.6				18.3	13.8			16.0			75.0																								
LOS	F			B				B	B			B			E																								
Approach Delay	105.9			11.6				14.4				74.4																											
Approach LOS	F			B				B				E																											
Queue Length 50th (m)	-101.5			3.1				10.0	44.4			1.3	-126.3																										
Queue Length 95th (m)	#162.6			10.3				20.8	60.4			5.1	#168.0																										
Internal Link Dist (m)	280.8			27.0				175.2				188.1																											
Turn Bay Length (m)								70.0				70.0																											
Base Capacity (vph)	508			509				260	1795			213	1291																										
Starvation Cap Reductn	0			0				0	0			0	0		0																								
Spillback Cap Reductn	0			0				0	0			0	0		0																								
Storage Cap Reductn	0			0				0	0			0	0		0																								
Reduced vlc Ratio	1.13			0.11				0.55	0.51			0.07	1.08																										

Intersection Summary

Area Type: Other

Cycle Length: 75

Actuated Cycle Length: 73

Natural Cycle: 100

Control Type: Actuated-Uncoordinated

Maximum vlc Ratio: 1.13

Intersection Signal Delay: 58.7

Intersection LOS: E

Intersection Capacity Utilization 97.5%

ICU Level of Service F

Analysis Period (min) 15

\* Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

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Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Post Development  
PM Peak Hour

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 2: Unwin Road & University Drive



# Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Post Development  
PM Peak Hour

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Post Development  
PM Peak Hour

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Post Development  
PM Peak Hour

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Post Development  
PM Peak Hour

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Post Development  
PM Peak Hour

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Unsignalized Intersection Capacity Analysis  
5: Uxbridge Drive & Lane2039 Post Development - Roundabouts  
AM Peak Hour

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑	↓	↑	↓	↑	↓
Volume (veh/h)	255	10	140	367	10	60
Sign Control	Free	Free	Stop			
Grade	0%	0%	0%	0%		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	
Hourly flow rate (vph)	268	11	147	386	11	63
Pedestrians	31		53	84		
Lane Width (m)	4.3		4.3	3.5		
Walking Speed (m/s)	1.2		1.2	1.2		
Percent Blockage	3		5	7		
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (m)						
px, platoon unblocked						
vC, conflicting volume		363	1070	411		
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol		363	1070	411		
IC, single (s)		4.5		6.4	6.2	
IC, 2 stage (s)						
If (s)		2.5		3.5	3.3	
p0 queue free %		85		94	89	
CM capacity (veh/h)		956		187	566	
Direction, Lane #	EB1	WB1	NB1			
Volume Total	279	534	74			
Volume Left	0	147	11			
Volume Right	11	0	63			
cSH	1700	956	439			
Volume to Capacity	0.16	0.15	0.17			
Queue Length 95th (m)	0.0	4.3	4.8			
Control Delay (s)	0.0	4.0	14.8			
Lane LOS	A	B				
Approach Delay (s)	0.0	4.0	14.8			
Approach LOS		B				
<b>Intersection Summary</b>						
Average Delay		3.6				
Intersection Capacity Utilization		64.8%	ICU Level of Service	C		
Analysis Period (min)		15				

## HCM Unsignalized Intersection Capacity Analysis

13: Tim Hortons &amp; Uxbridge Drive

## 2039 Post Development - Roundabouts

AM Peak Hour

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑	↑		
Volume (veh/h)	0	250	707	220	0	583
Sign Control	Stop	Free	Free	Free		
Grade	0%	0%	0%	0%		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	
Hourly flow rate (vph)	0	263	744	232	0	614
Pedestrians		128				
Lane Width (m)		3.5				
Walking Speed (m/s)		1.2				
Percent Blockage		10				
Right turn flare (veh)						
Median type		None		None		
Median storage (veh)						
Upstream signal (m)		83				
px, platoon unblocked						
vC, conflicting volume		1602	616	1104		
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol		1602	616	1104		
IC, single (s)		6.8	6.9	4.1		
IC, 2 stage (s)						
If (s)		3.5	3.3	2.2		
p0 queue free %		100	32	100		
CM capacity (veh/h)		87	389	563		
Direction, Lane #	WB1	NB1	NB2	SB1		
Volume Total	263	496	480	614		
Volume Left	0	0	0	0		
Volume Right	263	0	232	0		
cSH	389	1700	1700	1700		
Volume to Capacity	0.68	0.29	0.28	0.36		
Queue Length 95th (m)	38.5	0.0	0.0	0.0		
Control Delay (s)	31.6	0.0	0.0	0.0		
Lane LOS	D					
Approach Delay (s)	31.6	0.0	0.0			
Approach LOS	D					
<b>Intersection Summary</b>						
Average Delay		4.5				
Intersection Capacity Utilization		51.4%	ICU Level of Service	A		
Analysis Period (min)		15				

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TIASynchro 7 - Report  
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7: 16 Avenue & Uxbridge Drive2039 Post Development - Roundabouts  
AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑↑	↓	↑↑↑	↓	↑↑↑	↑↑↑	↓	↑↑↑	↑↑↑		
Volume (vph)	225	1580	790	370	1325	489	150	214	280	145		102
Ideal Flow (vphpl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850		1850
Lane Width (m)	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8	3.5		4.8
Storage Length (m)	85.0		85.0	75.0		0.0	110.0		0.0	50.0		0.0
Storage Lanes	1		1	2		0	2		0	1		0
Taper Length (m)	7.5		7.5	7.5		7.5	7.5		7.5	7.5		7.5
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	0.91	0.97	0.95	1.00	0.95		0.95
Ped/Bike Factor	0.98		0.74	0.96	0.94		0.82	0.88	0.93	0.92		
Fit		0.850		0.960			0.915			0.965		
Fit Protected	0.950		0.950			0.950			0.950			
Satd. Flow (prot)	1704	4802	1638	3122	4389	0	3306	2720	0	1704	2856	0
Fit Permitted	0.950		0.950			0.950			0.180			
Satd. Flow (perm)	1677	4802	1209	2991	4389	0	2725	2720	0	302	2856	0
Right Turn on Red	Yes		Yes			Yes			Yes			
Satd. Flow (RTOR)		341		70			195			23		
Link Speed (vph)		70		70			50			50		
Link Distance (m)		328.0		201.4			258.5			82.8		
Travel Time (s)		16.9		10.4			18.6			6.0		
Conf. Peds (#/hr)	120		134	134		120	246		109	109		246
Conf. Bikes (#/hr)			5			5			5	5		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95		0.95
Heavy Vehicles (%)	2%	4%	8%	2%	3%	2%	3%	2%	10%	2%		
Bus Blockages (#/hr)	0	0	11	0	0	6	0	0	0	0		0
Adj. Flow (vph)	237	1663	832	389	1395	515	158	225	295	153	354	107
Shared Lane Traffic (%)												
Lane Group Flow (vph)	237	1663	832	389	1910	0	158	520	0	153	461	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Right	
Median Width(m)	7.0		7.0			7.0			7.0			
Link Offset(m)	0.0		0.0			0.0			0.0			
Crosswalk Width(m)	4.8		4.8			4.8			4.8			
Two Way Left Turn Lanes												
Headway Factor	1.05	1.05	0.93	1.05	1.05	0.88	1.05	1.05	0.88	1.05	1.05	0.88
Turning Speed (vph)	25		15	25		15	25		15	25		15
Number of Detectors	1	1	1	1	1	1	1	1	1	1		
Detector Template	Left	Thru	Right	Left	Thru	Left	Thru	Left	Thru	Left	Thru	
Leading Detector (m)	8.0	4.0	8.0	4.0	8.0	4.0	8.0	4.0	8.0	4.0		
Trailing Detector (m)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		
Detector 1 Position(m)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		
Detector 1 Size(m)	6.0	2.0	2.0	6.0	2.0	6.0	2.0	6.0	2.0	6.0		
Detector 1 Type	Cl+Ex											
Detector 1 Channel												
Detector 1 Extent (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Turn Type	Prot	Perm	Prot			Prot			pm+pt			
Protected Phases	5	2	1	6	3	8			7	4		
Permitted Phases				2			4					

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Lane Group	5	2	2	1	6	WBL	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase												
Switch Phase												
Minimum Initial (s)	7.0	29.0	29.0	7.0	29.0							
Minimum Split (s)	11.5	35.0	35.0	11.5	35.0							
Total Split (s)	34.0	63.0	63.0	27.0	56.0	0.0	25.0	30.0	0.0	25.0	30.0	0.0
Total Split (%)	23.4%	43.4%	43.4%	18.6%	38.6%	0.0%	17.2%	20.7%	0.0%	17.2%	20.7%	0.0%
Maximum Green (s)	29.5	57.0	57.0	22.5	50.0	20.0	24.0			22.0	24.0	
Yellow Time (s)	3.5	4.5	4.5	3.5	4.5					3.0	3.5	
All Red Time (s)	1.0	1.5	1.5	1.0	1.5					2.0	2.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	6.0	4.5	6.0	4.0	5.0	6.0	4.0	5.0	6.0	4.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lag	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0					3.0	3.0	
Recall Mode	None	Min	Min	None	Min					None	None	
Walk Time (s)	10.0		10.0		10.0							

Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

2039 Post Development - Roundabouts  
AM Peak Hour

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

Splits and Phases: 7: 16 Avenue & Uxbridge Drive



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Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Post Development - Roundabouts  
AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	212	16	121	29	11	21	121	1196	14	13	689	320
Ideal Flow (vphp)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	4.8	4.8	4.8	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Storage Length (m)	0.0	0.0	0.0	0.0	0.0	0.0	70.0	0.0	70.0	0.0	70.0	0.0
Storage Lanes	0	0	0	0	0	0	1	0	1	0	1	0
Turner Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	0.93						0.95	1.00		0.99	0.95	
Frt	0.953						0.954			0.998	0.952	
Frt Protected	0.971						0.977			0.950	0.950	
Satl. Flow (prot)	0	1759	0	0	0	0	1640	0	1704	3396	0	3081
Frt Permitted	0.776						0.805		0.128		0.178	
Satl. Flow (perm)	0	1373	0	0	0	0	1312	0	230	3396	0	3081
Right Turn on Red		Yes					Yes		Yes		Yes	
Satl. Flow (RTOR)	31						22			2		108
Link Speed (k/h)	50						50			50		50
Link Distance (m)	304.8						51.0			199.2		212.1
Travel Time (s)	21.9						3.7			14.3		15.3
Confli. Peds. (#/hr)	36						119		119			50
Confli. Bikes (#/hr)							5		5			5
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	3%	3%	3%
Bus Blockages (#/hr)	0	6	6	0	0	0	0	0	0	9	0	9
Adj. Flow (vph)	223	17					127	31	12	22	127	1259
Shared Lane Traffic (%)							127		1274	0	14	1062
Lane Group Flow (vph)	0	367	0	0	0	0	65	0	127	0	14	1062
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Right	Right
Median Width(m)	0.0						0.0			3.5		3.5
Link Offset(m)	0.0						0.0			0.0		0.0
Crosswalk Width(m)	4.8						4.8			4.8		4.8
Two way Left Turn Lane												
Headway Factor	0.88	0.91	0.88	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Turning Speed (k/h)	25			15	25		15	25		15	25	15
Number of Detectors	1	1			1	1		1	1		1	1
Detector Template	Left	Thru		Left	Thru		Left	Thru		Left	Thru	
Leading Detector (m)	2.0	4.0			2.0	4.0		8.0	4.0		8.0	4.0
Trailing Detector (m)	0.0	2.0			0.0	2.0		2.0	2.0		2.0	2.0
Detector 1 Position(m)	0.0	2.0			0.0	2.0		2.0	2.0		2.0	2.0
Detector 1 Size(m)	2.0	2.0			2.0	2.0		6.0	2.0		6.0	2.0
Detector 1 Type	Cl+Ex	Cl+Ex			Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0	0.0		0.0	0.0		0.0	0.0
Detector 1 Queue (s)	0.0	0.0			0.0	0.0		0.0	0.0		0.0	0.0
Detector 1 Delay (s)	0.0	0.0			0.0	0.0		0.0	0.0		0.0	0.0
Turn Type	Perm				Perm			pm+pt			Perm	
Protected Phases	4				8			5		2		6
Permitted Phases	4				8			2				6

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Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Post Development - Roundabouts  
AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8		5	2		6		6
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		4.0	18.5		18.5		18.5
Minimum Split (s)	28.0	28.0		28.0	28.0		9.0	26.5		26.5		26.5
Total Split (s)	34.0	34.0	0.0	34.0	34.0	0.0	12.0	56.0	0.0	44.0	0.0	44.0
Total Split (%)	37.8%	37.8%	0.0%	37.8%	37.8%	0.0%	13.3%	62.2%	0.0%	48.9%	0.0%	48.9%
Maximum Green (s)	29.0	29.0		29.0	29.0		9.0	51.5		39.5		39.5
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0		3.0
All-Red Time (s)	2.0	2.0		2.0	2.0		0.0	1.5		1.5		1.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	4.0	5.0	5.0	4.0	3.0	4.5	4.0	4.5	4.0	4.0
Lead/Lag				Lead			Lag			Lag		
Lead-Lag Optimize?	3.0	3.0		3.0	3.0		3.0	3.0		3.0		3.0
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0		3.0
Recall Mode	None	None		None	None		None	Min		Min		Min
Walk Time (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0		8.0
Flash Dont Walk (s)	15.0	15.0		15.0	15.0		13.0	13.0		13.0		13.0
Pedestrian Calls (#/hr)	8	8		2	2		90	16		16		16
Act Effct Green (s)	23.0			40.2	38.6		30.1	30.1				
Actuated g/C Ratio	0.32			0.32	0.56	0.54	0.42	0.42				
v/c Ratio	0.80			0.15	0.42	0.70	0.11	0.79				
Control Delay	37.5			15.9	12.3	14.7	17.6	22.3				
Queue Delay	0.0			0.0	0.0	0.0	0.0	0.0				
Total Delay	37.5			15.9	12.3	14.7	17.6	22.3				
LOS	D			B	B	B	B	C				
Approach Delay	37.5			15.9		14.5		22.3				
Approach LOS	D			B		B		C				
Queue Length 50th (m)	47.1			4.6	8.3	68.2	1.3	68.1				
Queue Length 95th (m)	#102.2			15.0	17.4	99.7	5.6	100.3				
Internal Link Dist (m)	280.8			27.0		175.2		188.1				
Turn Bay Length (m)							70.0	70.0				
Base Capacity (vph)	619			587	329	2470	188	1882				
Starvation Cap Reductn	0			0	0	0	0	0				
Spillback Cap Reductn	0			0	0	0	0	0				
Storage Cap Reductn	0			0	0	0	0	0				
Reduced v/c Ratio	0.59			0.11	0.39	0.52	0.07	0.56				
Intersection Summary												
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 71.9												
Natural Cycle: 65												
Control Type: Actuated-Uncoordinated												
Maximum v/c Ratio: 0.80												
Intersection Signal Delay: 20.3												
Intersection LOS: C												
Intersection Capacity Utilization: 91.0%												
Analysis Period (min) 15												
# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.												

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Lanes, Volumes, Timings  
2: Unwin Road & University Drive

2039 Post Development - Roundabouts  
AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
a2	34			34								



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HCM Unsignalized Intersection Capacity Analysis  
5: Uxbridge Drive & Lane

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Volume (veh/h)	268	3	40	286	3	80
Sign Control	Free	Stop	Free	Stop	Free	Free
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	282	3	42	301	3	84
Pedestrians	21		53	45		
Lane Width (m)	4.3		4.3	3.5		
Walking Speed (m/s)	1.2		1.2	1.2		
Percent Blockage	2		5	4		
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (m)						
px, platoon unblocked						
vC, conflicting volume	330		735	382		
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	330		735	382		
vC, single (s)	4.5		6.4	6.2		
vC, 2 stage (s)						
tf (s)	2.5		3.5	3.3		
p0 queue free %	96		99	86		
CM capacity (veh/h)	1018		350	607		
Direction, Lane #	EB1	WB1	NB1			
Volume Total	285	343	87			
Volume Left	0	42	3			
Volume Right	3	0	84			
cSH	1700	1018	592			
Volume to Capacity	0.17	0.04	0.15			
Queue Length 95th (m)	0.0	1.0	4.1			
Control Delay (s)	0.0	1.4	12.1			
Lane LOS	A	B				
Approach Delay (s)	0.0	1.4	12.1			
Approach LOS			B			
Intersection Summary						
Average Delay		2.2				
Intersection Capacity Utilization	55.1%		ICU Level of Service	B		
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis  
13: Tim Hortons & Uxbridge Drive

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Volume (veh/h)	0	50	690	90	0	865
Sign Control	Stop	Free	Free	Free		
Grade	0%	0%	0%	0%		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	0	53	726	95	0	911
Pedestrians	69					
Lane Width (m)	3.5					
Walking Speed (m/s)	1.2					
Percent Blockage	6					
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (m)			83			
px, platoon unblocked						
vC, conflicting volume	1753	480			890	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1753	480			890	
vC, single (s)	6.8	6.9	4.1			
vC, 2 stage (s)						
If (s)	3.5	3.3	2.2			
p0 queue free %	100	90	100			
CM capacity (veh/h)	72	503	715			
Direction, Lane #	WB1	NB1	NB2	SB1		
Volume Total	53	484	337	911		
Volume Left	0	0	0	0		
Volume Right	53	0	95	0		
cSH	503	1700	1700	1700		
Volume to Capacity	0.10	0.28	0.20	0.54		
Queue Length 95th (m)	2.8	0.0	0.0	0.0		
Control Delay (s)	13.0	0.0	0.0	0.0		
Lane LOS	B					
Approach Delay (s)	13.0	0.0	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay		0.4				
Intersection Capacity Utilization	50.1%		ICU Level of Service	A		
Analysis Period (min)	15					

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Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	251	1265	90	270	1932	243	410	286	560	318	260	287
Ideal Flow (vphfl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8
Storage Length (m)	85.0		85.0	75.0		0.0	110.0		0.0	50.0		0.0
Storage Lanes	1		1	2		0	2		1	1		0
Taper Length (m)	7.5		7.5	7.5		7.5	7.5		7.5	7.5		7.5
Lane Util. Factor	1.00	0.91	1.00	0.97	0.91	0.97	1.00	1.00	1.00	0.95	0.95	
Ped/Bike Factor	1.00		0.77	0.94	0.99		0.88		0.78	0.90	0.86	
Flt		0.850		0.983			0.850		0.921			
Flt Protected	0.950		0.950		0.950		0.950		0.950			
Sald. Flow (prot)	1704	4802	1638	3122	4748	0	3306	1776	1747	1704	2598	0
Flt Permitted	0.950		0.950		0.950		0.950		0.248			
Sald. Flow (perm)	1697	4802	1253	2938	4748	0	2921	1776	1367	401	2598	0
Right Turn on Red	Yes											
Sald. Flow (RTOR)	91		20				293				180	
Link Speed (vph)	70		70				50				50	
Link Distance (m)	328.0		201.4				258.5				82.8	
Travel Time (s)	16.9		10.4				18.6				6.0	
Conf. Peds (#/hr)	56		128	128	56	147	117	117	117	147		
Conf. Bikes (#/hr)		5		5			5				5	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	
Heavy Vehicles (%)	2%	4%	8%	2%	3%	2%	3%	2%	2%	10%	2%	
Bus Blockages (#/hr)	0	0	11	0	0	6	0	0	0	0	0	
Adj. Flow (vph)	264	1332	95	284	2034	256	432	301	589	335	274	302
Shared Lane Traffic (%)												
Lane Group Flow (vph)	264	1332	95	284	2030	0	432	301	589	335	576	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Thru	Right	Left	Left	Right
Median Width(m)	7.0											7.0
Link Offset(m)	0.0		0.0		0.0		0.0		0.0			
Crosswalk Length(m)	4.8				4.8		4.8		4.8			
Two Way Left Turn Lane												
Headway Factor	1.05	1.05	0.93	1.05	1.05	0.88	1.05	1.05	0.88	1.05	1.05	0.88
Turning Speed (vph)	25		15	25	15	25	25	15	25	15	25	15
Number of Detectors	1	1	1	1	1	1	1	1	1	1	1	1
Detector Template	Left	Thru	Right	Left	Thru	Left	Thru	Right	Left	Thru		
Leading Detector (m)	8.0	4.0	8.0	4.0	8.0	4.0	8.0	2.0	8.0	4.0		
Trailing Detector (m)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0	2.0	2.0		
Detector 1 Position(m)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	0.0	2.0		
Detector 1 Size(m)	6.0	2.0	2.0	6.0	2.0	6.0	2.0	2.0	6.0	2.0		
Detector 1 Type	Cl+Ex											
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue(s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Turn Type	Prot	Perm	Prot	Prot	Prot	Perm	Perm	pm+pt	Prot	Prot		
Protected Phases	5	2	1	6	3	8	7	4	8	4		
Permitted Phases												

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase												
Switch Phase	5	2	2	1	6	3	8	8	8	7	4	
Minimum Initial (s)	21.0	29.0	29.0	7.0	29.0	7.0	17.0	17.0	17.0	10.0	10.0	29.0
Minimum Split (s)	11.5	35.0	35.0	11.5	35.0	12.0	29.0	29.0	13.0	29.0	29.0	
Total Split (s)	21.0	51.0	51.0	30.0	60.0	0.0	25.0	32.0	32.0	22.0	29.0	0.0
Total Split (%)	15.6%	37.8%	37.8%	22.2%	44.4%	0.0%	18.5%	23.7%	23.7%	16.3%	21.5%	0.0%
Maximum Green (s)	16.5	45.0	45.0	25.5	54.0	20.5	26.0	26.0	19.0	23.0		
Yellow Time (s)	3.5	4.5	4.5	3.5	4.5	3.0	3.5	3.5	3.0	3.5	3.0	3.5
All-Red Time (s)	1.0	1.5	1.5	1.0	1.5	1.5	2.5	2.5	2.5	0.0	0.0	2.5
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	6.0	4.5	6.0	4.0	4.5	6.0	6.0	3.0	6.0	
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lead	Lag	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.										

### Lanes, Volumes, Timings 7: 16 Avenue & Uxbridge Drive

### 2039 Post Development - Roundabouts PM Peak Hour

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

Splits and Phases: 7: 16 Avenue & Uxbridge Drive



### Lanes, Volumes, Timings 2: Unwin Road & University Drive

### 2039 Post Development - Roundabouts PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	400	16	144	19	11	21	127	845	24	13	1064	265
Ideal Flow (vphp)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	4.8	4.8	4.8	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5	3.5
Storage Length (m)	0.0	0.0	0.0	0.0	0.0	70.0	0.0	0.0	0.0	70.0	0.0	0.0
Storage Lanes	0	0	0	0	0	1	0	1	0	1	0	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00	0.95
Ped Bike Factor	0.87						0.92				0.98	0.98
Frt	0.965						0.945				0.996	
Flt Protected	0.966						0.982				0.950	
Flt Protected	0	1836	0	0	1540	0	1704	3386	0	1704	3205	0
Flt Permitted	0.754						0.801				0.075	
Flt Permitted	0	1268	0	0	1250	0	135	3386	0	473	3205	0
Right Turn on Red							Yes			Yes		Yes
Salid. Flow (RTOR)	18						22			3		32
Link Speed (k/h)	50						50			50		50
Link Distance (m)	304.8						51.0			199.2		212.1
Travel Time (s)	21.9						3.7			14.3		15.3
Confli. Peds. (#/hr)	111						37			29		29
Confli. Bikes (#/hr)							5			5		5
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	2%	2%	2%	2%	2%	2%	2%	2%	2%	3%	3%
Bus Blockages (#/hr)	0	6	6	0	0	0	0	0	0	9	0	9
Adj. Flow (vph)	421	17	152	20	12	22	134	889	25	14	1120	279
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	590	0	0	54	0	134	914	0	14	1399	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Right	Right
Median Width(m)	0.0						0.0			3.5		3.5
Link Offset(m)	0.0						0.0			0.0		0.0
Crosswalk Width(m)	4.8						4.8			4.8		4.8
Two way Left Turn Lane												
Headway Factor	0.88	0.91	0.88	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05	1.05
Turning Speed (k/h)	25						15			25		15
Number of Detectors	1	1			1	1			1	1		
Detector Template	Left	Thru			Left	Thru			Left	Thru		
Leading Detector (m)	2.0	4.0			2.0	4.0			8.0	4.0		
Trailing Detector (m)	0.0	2.0			0.0	2.0			2.0	2.0		
Detector 1 Position(m)	0.0	2.0			0.0	2.0			2.0	2.0		
Detector 1 Size(m)	2.0	2.0			2.0	2.0			6.0	2.0		
Detector 1 Type	Cl+Ex	Cl+Ex			Cl+Ex	Cl+Ex			Cl+Ex	Cl+Ex		
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0	0.0			0.0	0.0		
Detector 1 Queue (s)	0.0	0.0			0.0	0.0			0.0	0.0		
Detector 1 Delay (s)	0.0	0.0			0.0	0.0			0.0	0.0		
Turn Type	Perm				Perm				pm+pt		Perm	
Protected Phases	4						8		5	2		6
Permitted Phases	4						8		2			6

### Lanes, Volumes, Timings 2: Unwin Road & University Drive

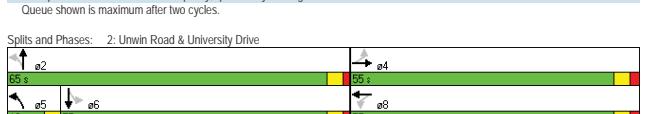
### 2039 Post Development - Roundabouts PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8		5	2		6		
Switch Phase												
Minimum Initial (s)	10.0	10.0		10.0	10.0		4.0	18.5		18.5		
Minimum Split (s)	28.0	28.0		28.0	28.0		9.0	26.5		26.5		
Total Split (s)	55.0	55.0	0.0	55.0	55.0	0.0	10.0	65.0	0.0	55.0	0.0	
Total Split (%)	45.8%	45.8%	0.0%	45.8%	45.8%	0.0%	8.3%	54.2%	0.0%	45.8%	45.8%	0.0%
Maximum Green (s)	50.0	50.0		50.0	50.0		7.0	60.5		50.5		
Yellow Time (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0		
All-Red Time (s)	2.0	2.0		2.0	2.0		0.0	1.5		1.5		
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	5.0	5.0		5.0	5.0		4.0	3.0	4.5	4.5	4.0	
Lead/Lag							Lead	Lag	Lag	Lag		
Lead-Lag Optimize?	3.0	3.0		3.0	3.0		3.0	3.0		3.0		
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0		
Recall Mode	None	None		None	None		None	Min	Min	Min		
Walk Time (s)	8.0	8.0		8.0	8.0		8.0	8.0		8.0		
Flash Don't Walk (s)	15.0	15.0		15.0	15.0		13.0	13.0	13.0	13.0		
Pedestrian Calls (#/hr)	8	8		2	2		90	16	16	16		
Act Eff Green (s)	50.0			50.0			62.0	60.5	50.5	50.5		
Actuated g/C Ratio	0.42			0.42			0.52	0.50	0.42	0.42		
v/c Ratio	1.09			0.10			0.83	0.53	0.07	1.02		
Control Delay	100.4			14.7			59.7	21.6	22.2	64.4		
Queue Delay	0.0			0.0			0.0	0.0	0.0	0.0		
Total Delay	100.4			14.7			59.7	21.6	22.2	64.4		
LOS	F			B			E	C	E	E		
Approach Delay	100.4			14.7			26.4			64.0		
Approach LOS	F			B			C			E		
Queue Length 50th (m)	-163.0			4.7			16.8	78.1	2.0	-191.7		
Queue Length 95th (m)	#235.4			13.2			#52.8	97.2	6.7	#236.7		
Internal Link Dist (m)	280.8			27.0			175.2			188.1		
Turn Bay Length (m)				70.0			70.0					
Base Capacity (vph)	539			534			161	1709	199	1367		
Starvation Cap Reductn	0			0			0	0	0	0		
Spillback Cap Reductn	0			0			0	0	0	0		
Storage Cap Reductn	0			0			0	0	0	0		
Reduced v/c Ratio	1.09			0.10			0.83	0.53	0.07	1.02		

\* Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

### Splits and Phases: 2: Unwin Road & University Drive



### Lanes, Volumes, Timings 7: 16 Avenue & Uxbridge Drive

2039 Post Development Improved  
AM Peak Hour

Lane Group	EBL	EBT	EBr	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑	↑↑↑	↑↑↑	↑↑	↑↑↑	↑↑↑	↑↑↑	↑↑	↑↑↑	↑↑↑	↑↑	↑↑↑
Volume (vph)	225	1580	790	370	1325	369	150	214	280	145	336	102
Ideal Flow (vphpl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8
Storage Length (m)	85.0	85.0	75.0	0.0	110.0	0.0	50.0	0.0	50.0	0.0	50.0	0.0
Storage Lanes	2		1	2			0	2		0	1	0
Taper Length (m)	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5	7.5
Lane Util. Factor	0.97	0.91	1.00	0.97	0.91	0.91	0.97	0.95	0.95	1.00	0.95	0.95
Ped/Bike Factor	0.97	0.74	0.96	0.95		0.82	0.88		0.93	0.92		
Frt		0.850	0.967			0.915			0.965			
Fit Protected	0.950		0.950			0.950			0.950			
SaId. Flow (prot)	3306	4802	1638	3122	4482	0	3306	2720	0	1704	2856	0
Fit Permitted	0.950		0.950			0.950			0.208			
SaId. Flow (perm)	3200	4802	1209	2991	4482	0	2725	2720	0	348	2856	0
Right Turn on Red	Yes		Yes			Yes			Yes		Yes	
SaId. Flow (RTOR)		382		56			195			23		
Link Speed (k/h)	70			70			50			50		
Link Distance (m)	328.0			201.4			258.5			82.8		
Travel Time (s)	16.9			10.4			18.6			6.0		
Conf. Peds. (#/hr)	120		134	134		120	246		109	109		246
Conf. Bikes (#/hr)			5			5			5			5
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	4%	4%	8%	2%	3%	2%	3%	2%	2%	10%	2%
Bus Blockages (#/hr)	0	0	11	0	0	6	0	0	0	0	0	0
Adj. Flow (vph)	237	1663	832	389	1395	388	158	225	295	153	354	107
Shared Lane Traffic (%)												
Lane Group Flow (vph)	237	1663	832	389	1783	0	158	520	0	153	461	0
Enter Blocked Intersection	No	No										
Lane Alignment	Left	Left	Right	Left	Right	Left	Left	Right	Left	Left	Right	
Median Width(m)	7.0			7.0			7.0			7.0		
Link Offset(m)	0.0		0.0			0.0			0.0			
Crosswalk Width(m)	4.8			4.8			4.8			4.8		
Two way Left Turn Lane												
Headway Factor	1.05	1.05	0.93	1.05	1.05	0.88	1.05	1.05	0.88	1.05	1.05	0.88
Turning Speed (k/h)	25		15	25		15	25		15	25		15
Number of Detectors	1	1	1	1	1		1	1	1	1		
Detector Template	Left	Thru	Right	Left	Thru	Left	Thru	Left	Thru	Left	Thru	
Leading Detector (m)	8.0	4.0	4.0	8.0	4.0		8.0	4.0	8.0	4.0		
Trailing Detector (m)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0		
Detector 1 Position(m)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0		
Detector 1 Size(m)	6.0	2.0	2.0	6.0	2.0		6.0	2.0	6.0	2.0		
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		
Turn Type	Prot		Perm	Prot			Prot			pm+pt		
Protected Phases	5	2	1	6		3	8		7	4		
Permitted Phases				2				4				

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### Lanes, Volumes, Timings 7: 16 Avenue & Uxbridge Drive

2039 Post Development Improved  
AM Peak Hour

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 7: 16 Avenue & Uxbridge Drive

Splits and Phases: 7: 16 Avenue & Uxbridge Drive



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### Lanes, Volumes, Timings 7: 16 Avenue & Uxbridge Drive

2039 Post Development Improved  
AM Peak Hour

Lane Group	EBL	EBT	EBr	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	5	2	1	6	0	1	3	8	7	4		
Switch Phase												
Minimum Initial (s)	7.0	29.0	29.0	7.0	29.0	7.0	17.0	17.0	10.0	10.0		
Minimum Split (s)	11.5	35.0	35.0	11.5	35.0	12.0	29.0	29.0	13.0	29.0		
Total Split (s)	26.0	55.0	55.0	32.0	61.0	0.0	28.0	30.0	0.0	28.0	30.0	0.0
Total Split (%)	17.9%	37.9%	37.9%	22.1%	42.1%	0.0%	19.3%	20.7%	0.0%	19.3%	20.7%	0.0%
Maximum Green (s)	21.5	49.0	49.0	27.5	55.0	23.0	24.0	25.0	24.0	25.0	24.0	
Yellow Time (s)	3.5	4.5	4.5	3.5	4.5	3.0	3.5	3.0	3.5	3.0	3.5	
All-Red Time (s)	1.0	1.5	1.5	1.0	1.5	2.0	2.5	0.0	0.0	0.0	2.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	6.0	4.5	6.0	4.0	5.0	4.0	3.0	6.0	4.0	4.0
Lead/Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Recall Mode	None	Min	Min	None	Min	None	None	None	None	None	None	None
Walk Time (s)	10.0	10.0	10.0	10.0	10.0		8.0		8.0		8.0	
Flash Don't Walk (s)	13.0	13.0	13.0	13.0	13.0		15.0		15.0		15.0	
Pedestrian Calls (#/hr)	104	104		74		104			104			13
Act Effct Green (s)	14.3	49.5	49.5	20.9	56.0	11.3	23.8	41.7	24.6			
Actuated g/C Ratio	0.11	0.39	0.39	0.16	0.44	0.09	0.19	0.33	0.19			
W/C Ratio	0.64	0.89	1.19	0.76	0.89	0.54	0.78	0.58	0.81			
Control Delay	63.0	44.8	117.8	61.8	39.7	63.7	40.4	40.2	59.8			
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Total Delay	63.0	44.8	117.8	61.8	39.7	63.7	40.4	40.2	59.8			
LOS	E	D	F	E	D	E	E	D	D	E		
Approach Delay	68.6			43.6			45.9			54.9		
Approach LOS	E			D			D			D		
Queue Length 50th (m)	31.0	148.0	-197.3	50.7	151.3	20.7	43.9	28.9	58.7			
Queue Length 95th (m)	47.9	#212.6	#305.9	72.3	#214.3	34.7	#76.8	49.7	#90.4			
Internal Link Dist (m)	304.0					177.4				234.5		58.8
Turn Bay Length (m)	85.0										50.0	
Base Capacity (vph)	558	1859	702	674	1995	597	671	386	573			
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0		
Reduced v/C Ratio	0.42	0.89	1.19	0.58	0.89	0.26	0.77	0.40	0.80			

Intersection Summary

Area Type:

Cycle Length: 145

Actuated Cycle Length: 127.8

Natural Cycle: 140

Control Type: Actuated-Uncoordinated

Maximum W/C Ratio: 1.19

Intersection Signal Delay: 56.0

Intersection Capacity Utilization 100.8%

ICU Level of Service G

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.

- Queue shown is maximum after two cycles.

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### Lanes, Volumes, Timings 7: 16 Avenue & Uxbridge Drive

2039 Post Development Improved  
PM Peak Hour

Lane Group	EBL	EBT	EBr	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑	↑↑↑↑
Volume (vph)	251	1265	90	270	1932	213	410	286	560	318	260	287
Ideal Flow (vphpl)	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850	1850
Lane Width (m)	3.5	3.5	4.8	3.5	3.5	4.8	3.5	3.5	4.8	3.5	4.8	3.5
Storage Length (m)	85.0	85.0	75.0	0.0	110.0	0.0	50.0	0.0</				

Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

2039 Post Development Improved  
PM Peak Hour

Lane Group	EBL	EBT	EVR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	5	2	2	1	6							
Switch Phase												
Minimum Initial (s)	7.0	29.0	29.0	7.0	29.0		7.0	17.0	17.0	10.0	10.0	
Minimum Split (s)	11.5	35.0	35.0	11.5	35.0		12.0	29.0	29.0	13.0	29.0	
Total Split (s)	15.0	61.0	61.0	22.0	68.0	0.0	23.0	35.0	35.0	17.0	29.0	0.0
Total Split (%)	11.1%	45.2%	45.2%	16.3%	50.4%	0.0%	17.0%	25.9%	25.9%	12.6%	21.5%	0.0%
Maximum Green (s)	10.5	55.0	55.0	17.5	62.0		18.0	29.0	29.0	14.0	23.0	
Yellow Time (s)	3.5	4.5	4.5	3.5	4.5		3.0	3.5	3.5	3.0	3.5	
All-Red Time (s)	1.0	1.5	1.5	1.0	1.5		2.0	2.5	2.5	0.0	2.5	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.5	6.0	6.0	4.5	6.0	4.0	5.0	6.0	6.0	3.0	6.0	4.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag		Lead	Lag	Lag	Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	Min	Min	None	Min		None	None	None	None	None	
Walk Time (s)	10.0	10.0	10.0				8.0	8.0	8.0			
Flash Don't Walk (s)	13.0	13.0	13.0				15.0	15.0	15.0			
Pedestrian Calls (#/hr)	104	104	74				104	104	13			
Act Effct Green (s)	10.5	56.4	56.4	16.1	62.0		18.0	29.0	29.0	40.0	23.0	
Actuated g/C Ratio	0.08	0.42	0.42	0.12	0.46		0.13	0.21	0.21	0.30	0.17	
V/C Ratio	1.03	0.66	0.16	0.76	1.03		0.98	0.79	1.33	1.18	1.03	
Control Delay	123.3	33.9	5.4	71.4	62.3		96.0	65.8	192.8	146.9	86.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay	123.3	33.9	5.4	71.4	62.3		96.0	65.8	192.8	146.9	86.5	
LOS	F	C	A	E	E		F	E	F	F	F	
Approach Delay	46.2			63.3			132.3			108.7		
Approach LOS	D			E			F			F		
Queue Length 50th (m)	-40.6	110.8	0.0	39.9	-246.3		63.1	80.6	-171.0	-81.1	-72.4	
Queue Length 95th (m)	#70.1	128.6	11.3	55.9	#276.2		#98.3	#123.6	#246.1	#142.2	#111.6	
Internal Link Dist (m)					177.4			234.5			68.4	
Turn Bay Length (m)	85.0	85.0	75.0				110.0			50.0		
Base Capacity (vph)	257	2007	579	405	2198		441	382	442	283	560	
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	
Reduced v/c Ratio	1.03	0.66	0.16	0.70	1.03		0.98	0.79	1.33	1.18	1.03	

Intersection Summary

Area Type: Other  
Cycle Length: 135  
Actuated Cycle Length: 135  
Natural Cycle: 140  
Control Type: Actuated-Uncoordinated  
Maximum v/c Ratio: 1.33  
Intersection Signal Delay: 79.4  
Intersection LOS: E  
Intersection Capacity Utilization 105.3%  
Analysis Period (min) 15  
- Volume exceeds capacity, queue is theoretically infinite.  
Queue shown is maximum after two cycles.

Stadium Shopping Centre  
TIA

Synchro 7 - Report  
Page 2

Lanes, Volumes, Timings  
7: 16 Avenue & Uxbridge Drive

2039 Post Development Improved  
PM Peak Hour

# 95th percentile volume exceeds capacity, queue may be longer.  
Queue shown is maximum after two cycles.

Splits and Phases: 7: 16 Avenue & Uxbridge Drive



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Synchro 7 - Report  
Page 3

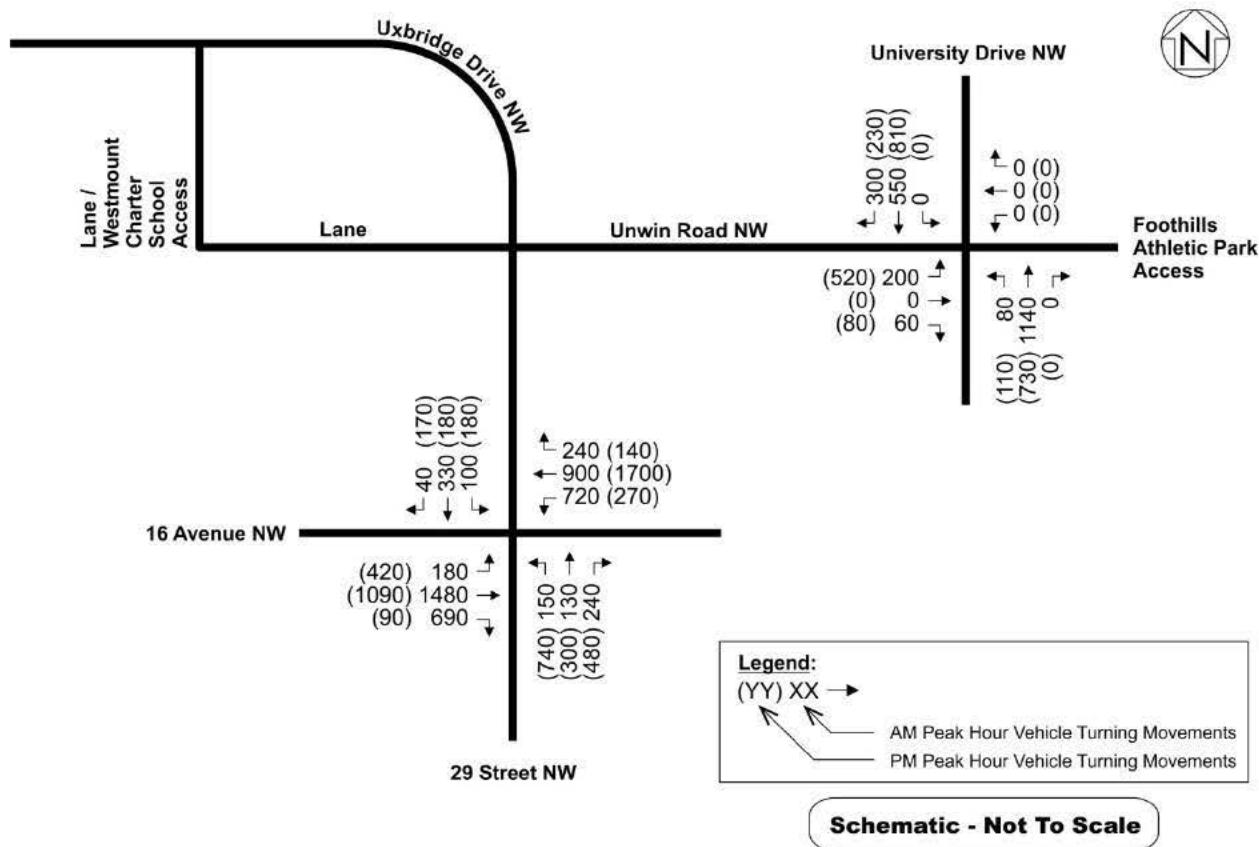
**APPENDIX F: 2039 MODE SPLIT DATA FROM RTM**

## 2039 Horizon TZ 2305 Mode Split

	AM Peak 2-Hour Person Trips by mode				PM Peak 2-Hour Person Trips by mode			
	Productions		Attractions		Productions		Attractions	
	TZ 2305	%	TZ 2305	%	TZ 2305	%	TZ 2305	%
SOV	498	33	421	29	577	41	712	40
HOV	360	24	369	25	506	36	571	32
Transit	237	16	268	18	142	10	217	12
Bike	25	2	15	1	12	1	23	1
Walk	367	25	377	26	172	12	261	15
<b>Totals</b>	<b>1487</b>	<b>100</b>	<b>1450</b>	<b>100</b>	<b>1409</b>	<b>100</b>	<b>1784</b>	<b>100</b>

## APPENDIX G: METHODOLOGY OF BACKGROUND TRAFFIC VOLUMES

The City of Calgary provided forecast turning movement volumes at the intersection of 16 Avenue / Uxbridge Drive and University Drive / Unwin Road. The RTM 2039 forecast traffic volumes are shown in **Figure A**.



**FIGURE A: RTM 2039 FORECAST TRAFFIC VOLUMES**

Using the forecast turning movement volumes at these two intersections and existing traffic counts, turning movement volumes were estimated at the remaining study intersections and balanced between intersections. Then a series of traffic volume adjustments were discussed with TDS and applied to the forecast traffic volumes to further refine the estimate of future traffic volumes as follows:

**Step 1** – Adjustments were applied based on congestion reassignment and a comparison to the existing traffic volumes. The adjustments are summarized in **Tables A, B, C and D**. Traffic volumes were again balanced between study intersections.

**TABLE A: 16 AVENUE / UXBRIDGE DRIVE AM PEAK HOUR ADJUSTMENTS**

<b>Adjustment</b>	<b>Reason</b>
- 350 WB left turns	Assume more traffic will use West Campus Boulevard interchange, no volume assigned to interchange in hospital TZ
+350 WB through	
+60 SB right	Increase forecast to meet existing traffic count
+40 SB through	Increase forecast to meet existing traffic count
+40 NB through	Increase forecast to meet existing traffic count
+40 NB right	No volume assigned to NB right from hospital TZ in forecast model
-40 EB through	

**TABLE B: 16 AVENUE / UXBRIDGE DRIVE PM PEAK HOUR ADJUSTMENTS**

<b>Adjustment</b>	<b>Reason</b>
-200 EB left	Growth on EB Left movement attributed to unrealistic short cutting in the model, reassign to 16 Avenue
+200 EB through	
+80 NB right	No volume assigned to NB right from hospital TZ in forecast model
-80 EB through	
-370 NB left	Assume more traffic will use West Campus Boulevard interchange
+70 SB right	Increase forecast to meet existing count
+30 SB through	Increase forecast to meet existing count

**TABLE C: UNIVERSITY DRIVE/ UNWIN ROAD AM PEAK HOUR ADJUSTMENTS**

<b>Adjustment</b>	<b>Reason</b>
+20 WB left	Volumes were set at zero, increase to match existing
+10 WB through	
+10 WB right	
+10 NB right	
+10 SB left	
+10 EB through	
+40 NB left	Increase to match existing

**TABLE D: UNIVERSITY DRIVE/ UNWIN ROAD PM PEAK HOUR ADJUSTMENTS**

Adjustment	Reason
+10 WB left	Volumes set at zero, increase to match existing
+10 WB through	
+10 WB right	
+20 NB right	
+10 SB left	
+10 EB through	
-200 EB left	Growth attributed to unrealistic short-cutting in the model, reassign to 16 avenue

**Step 2** – Adjustments were applied to account for the shortfall in estimated population and jobs in the West Campus TZ's. The following methodology was applied:

- Compare the west campus job and population estimates from the forecast model to the current understanding. This showed an 83% increase in density from the forecast model estimate to the current estimate.
- Using the total forecast trip generation from the two west campus zones (TZ 2403 and 2405) estimate the total additional trips to/from the west campus zones as 80% of the forecast model trips. (i.e. apply an 80% increase)
- Using the select zone plots for the west campus zones (**Appendix H**) calculate the increase in through volumes on each road. (16 Avenue and University Drive)

**Step 3** – Adjust for the impact of the Foothills Athletic Park redevelopment.

To estimate the impact of this development, TDS staff provided the turning movement volumes shown in **Figure B** to be added to the a.m. and p.m. peak hour future background traffic volumes at University Drive/ Unwin Road.


**FIGURE B: FOOTHILLS ATHLETIC CENTRE TURNING VOLUMES**

**Step 4 – Net out traffic volumes associated with the existing Stadium Shopping Centre.**

Using exiting traffic counts, the turning movement volumes associated with the existing shopping centre uses were netted out from the future background traffic volumes.

**Step 5 – All adjustment layers were summed together resulting in the final 2039 future background traffic volumes.**

Pedestrian and cyclist volumes were grown by 20 percent in the future background to account for general growth in pedestrian traffic outside Stadium Shopping Centre.

## APPENDIX H: WEST CAMPUS SELECT ZONE PLOTS

# R1523c1- West Campus TIA

## Select Zone Plots

Request Number:	R523c1
Location:	West Campus
Forecast Requested by:	Zoran Carkic
Company:	Development Services
Date Requested:	March 11, 2013
Forecast Prepared by:	Martin Kycak
Date Completed:	March 13, 2013
Cost:	N/A
Notes:	
Horizon:	2039 Target
Databases:	39P899_05_Target

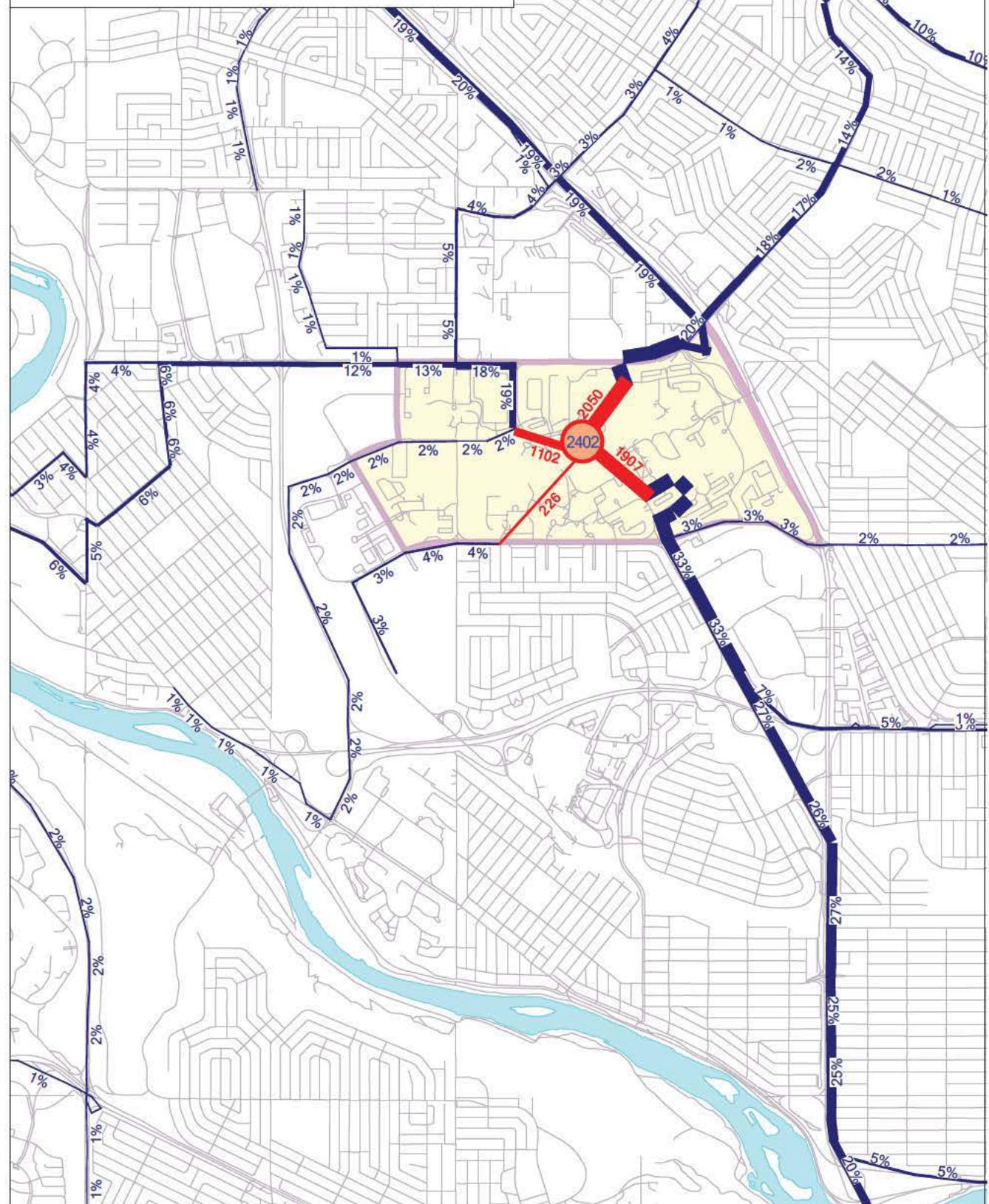
## Select Zone Area (TZ2402 - UofC)

Request: West Campus (R1523c1)

Horizon: 2039 Target

Time of day: AM Peak Hour - Inbound

\* Values less than 1% are suppressed for clarity.



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## Select Zone Area (TZ2402 - UofC)

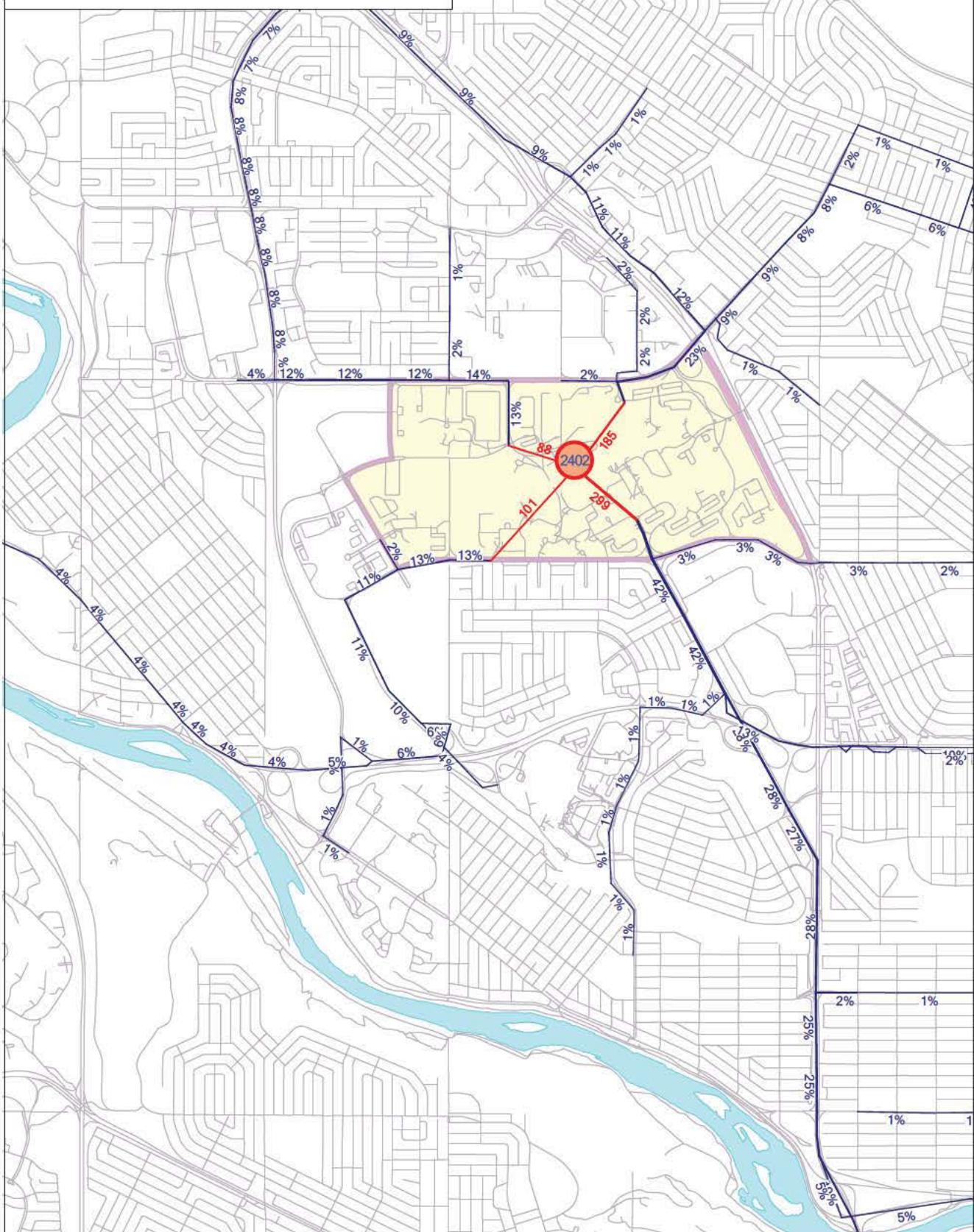
Request: West Campus (R1523c1)

Horizon: 2039 Target

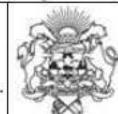
Time of day: AM Peak Hour - Outbound

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N



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## Select Zone Area (TZ2402 - UofC)

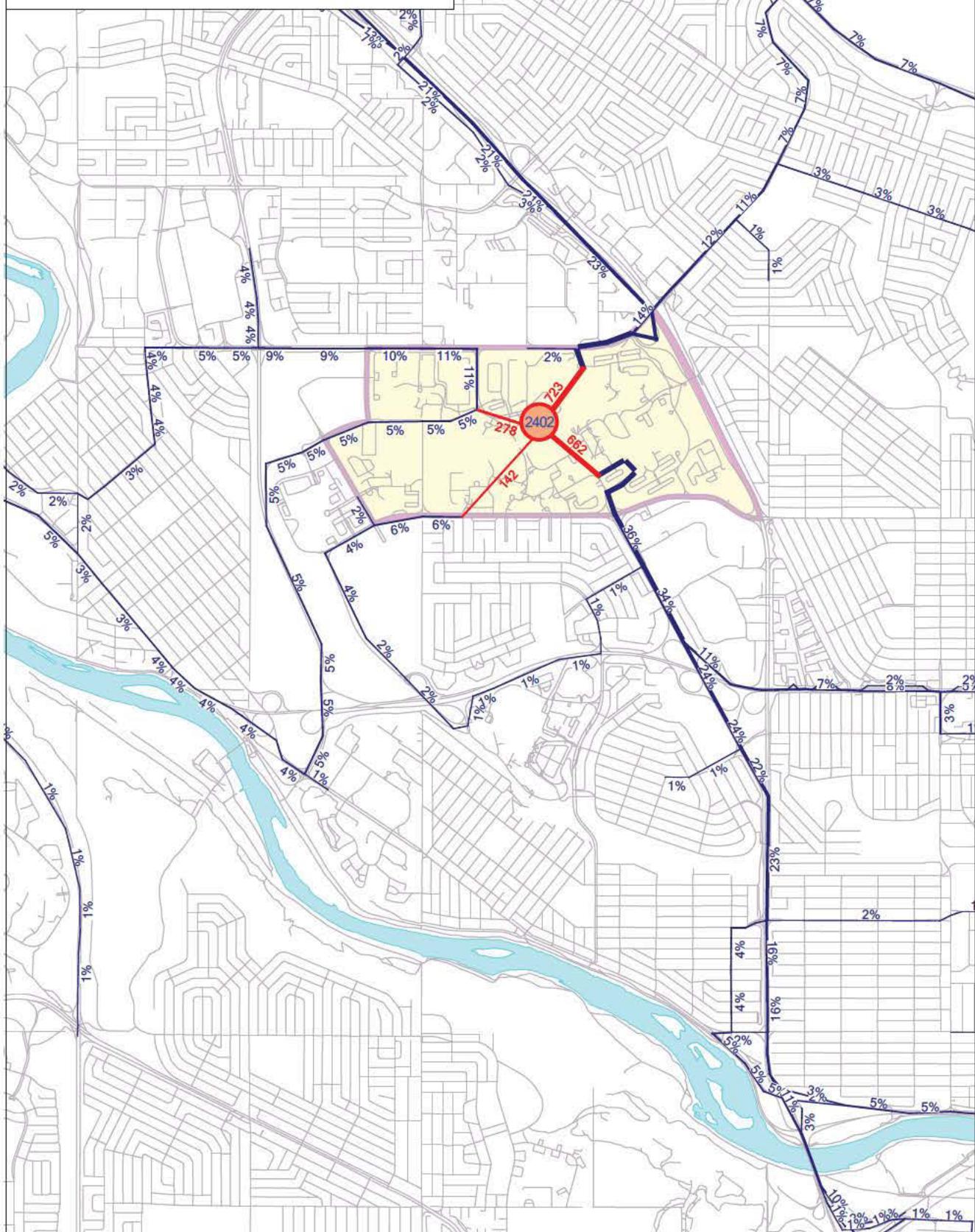
Request: West Campus (R1523c1)

Horizon: 2039 Target

Time of day: PM Peak Hour - Inbound

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## Select Zone Area (TZ2402 - UofC)

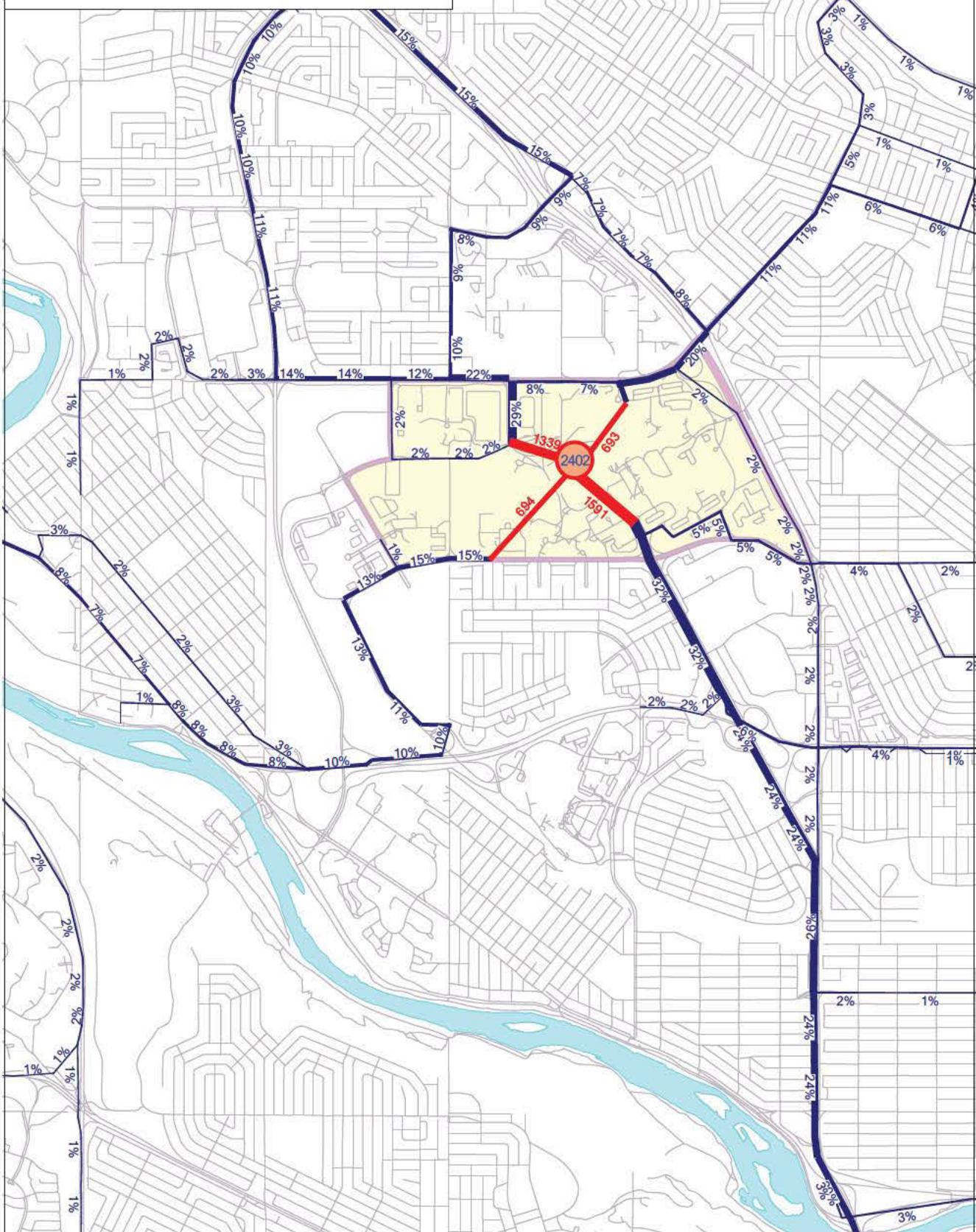
Request: West Campus (R1523c1)

Horizon: 2039 Target

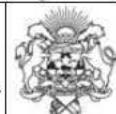
Time of day: PM Peak Hour - Outbound

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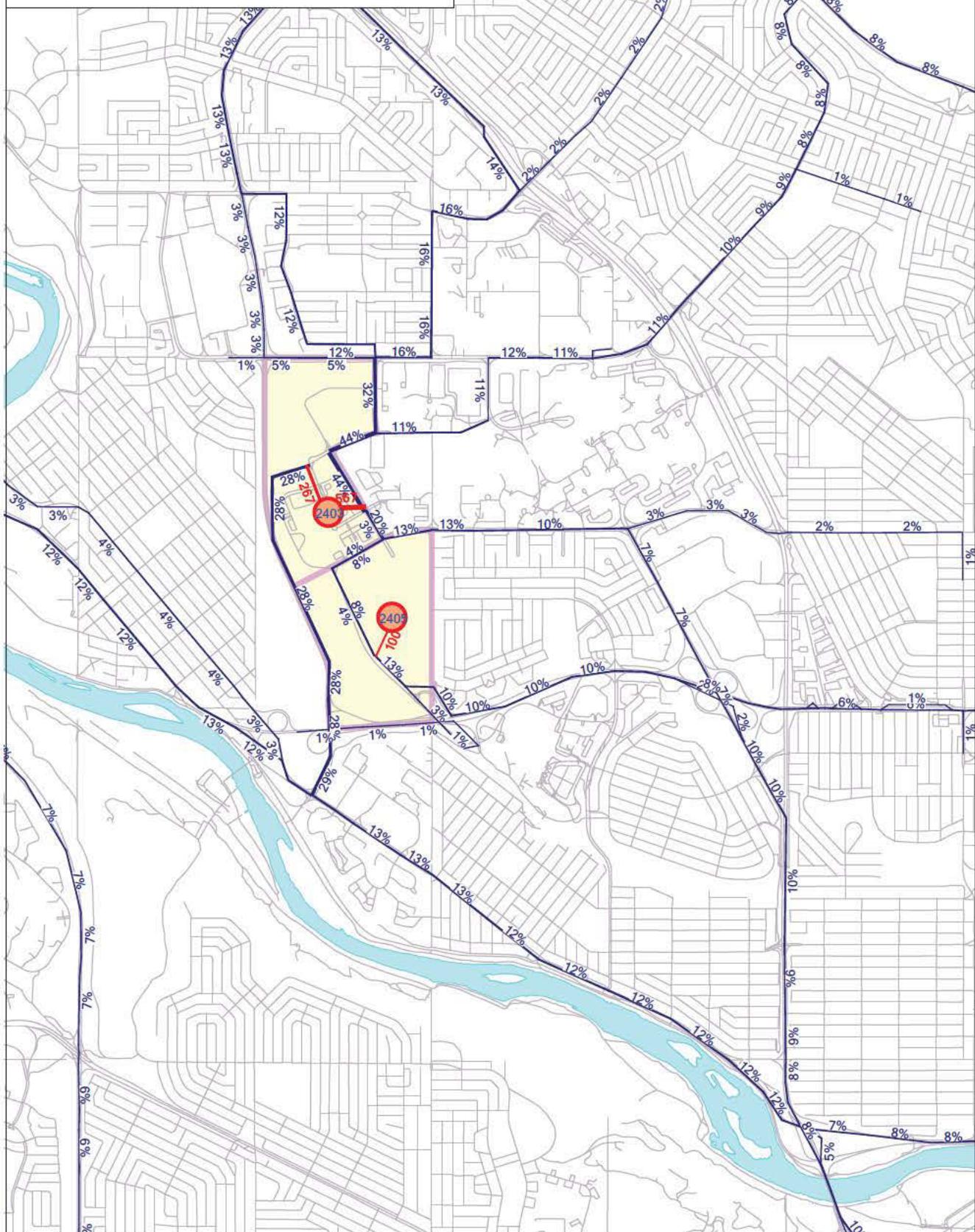
## Select Zone Area (TZ2403 & 2405)

Request: West Campus (R1523c1)

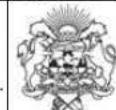
Horizon: 2039 Target

Time of day: AM Peak Hour - Inbound

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## Select Zone Area (TZ2403 & 2405)

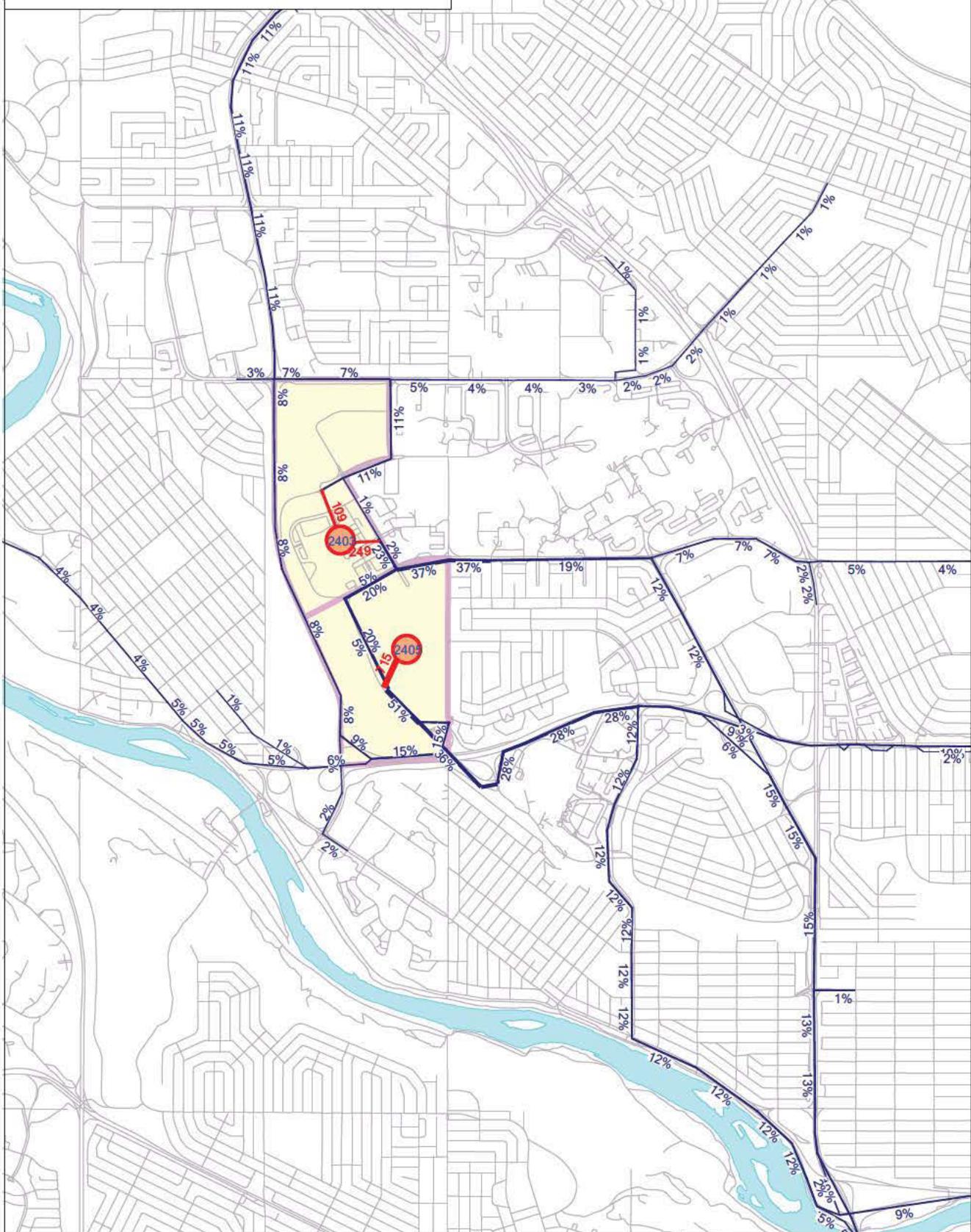
Request: West Campus (R1523c1)

Horizon: 2039 Target

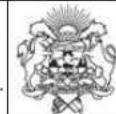
Time of day: AM Peak Hour - Outbound

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## Select Zone Area (TZ2403 & 2405)

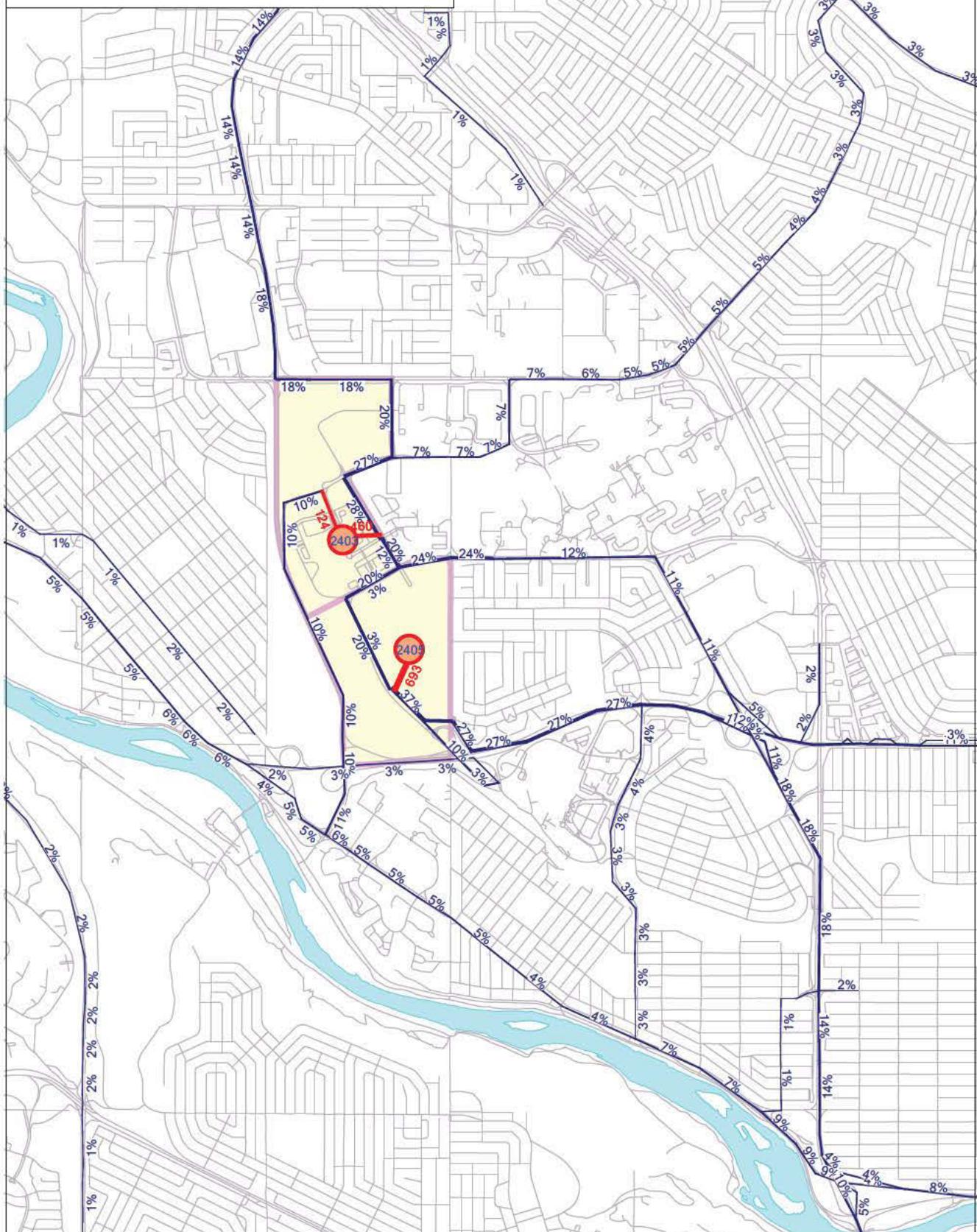
Request: West Campus (R1523c1)

Horizon: 2039 Target

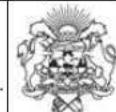
Time of day: PM Peak Hour - Inbound

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## Select Zone Area (TZ2403 & 2405)

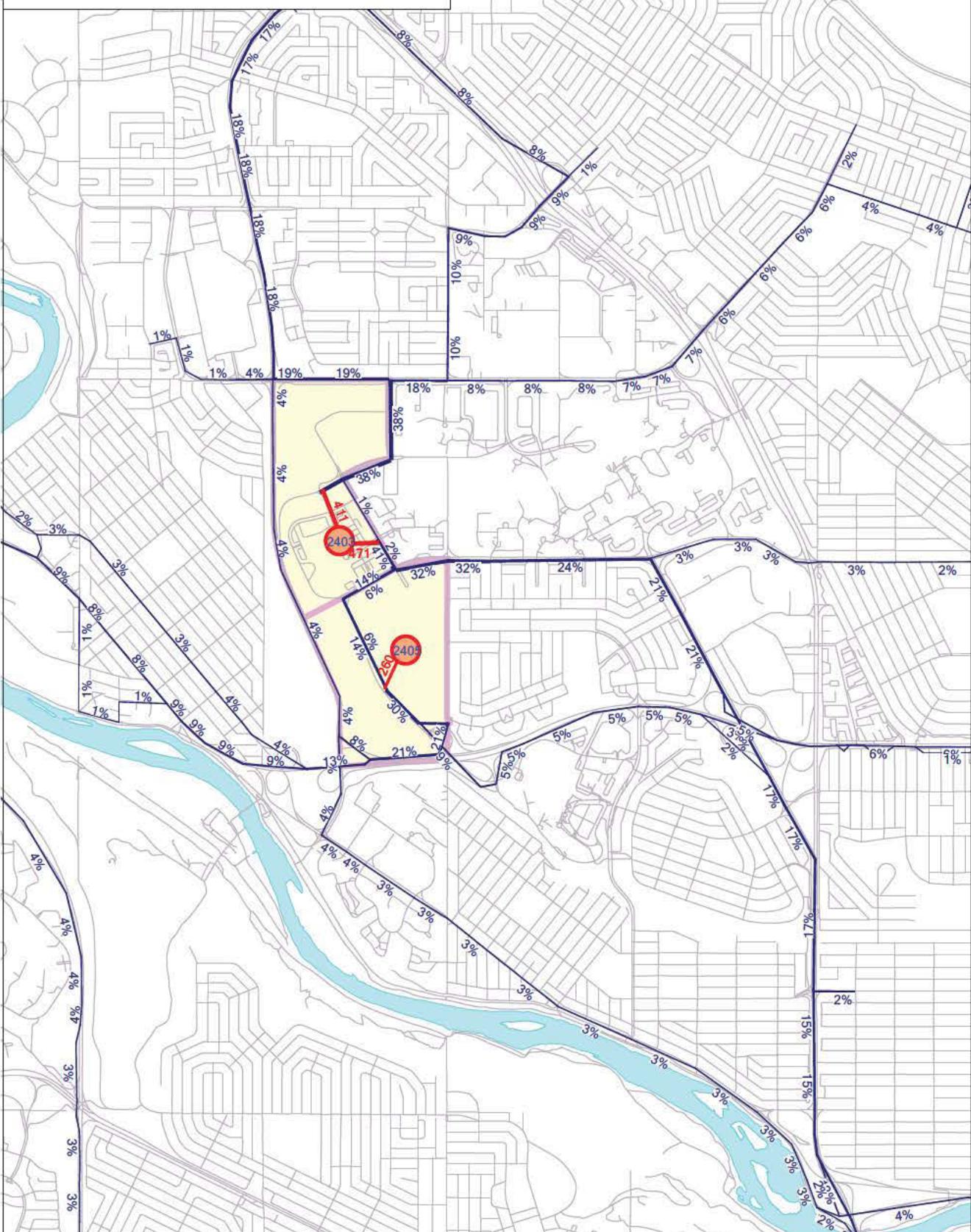
Request: West Campus (R1523c1)

Horizon: 2039 Target

Time of day: PM Peak Hour - Outbound

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## APPENDIX I: DETAILED TRIP RATE TABLES

Trip Rates

Land Use	Vehicle Trip Generation Rate									Source
	AM Peak Hour (IB/OB)			PM Peak Hour (IB/OB/PB)			Daily (IB/OB/PB)			
Office	1.39 / 1000 ft <sup>2</sup>	88%	12%	1.57 / 1000 ft <sup>2</sup>	17%	83%	/ 1000 ft <sup>2</sup>	50%	50%	City of Calgary - Suburban
Hotel	0.56 / unit	61%	39%	0.59 / unit	53%	47%	5.9 / unit	50%	50%	ITE - 310
Seniors	0.35 / unit	25%	75%	0.45 / unit	65%	35%	/ unit	50%	50%	City of Calgary TOD
Residential	0.35 / unit	25%	75%	0.45 / unit	65%	35%	/ unit	50%	50%	City of Calgary TOD
Restaurant	11.52 / 1000 ft <sup>2</sup>	52%	48%	11.15 / 1000 ft <sup>2</sup>	59%	41%	127.15 / 1000 ft <sup>2</sup>	50%	50%	ITE 932 - Highturnover (sit-down) restaurant
Medical Office	3 / 1000 ft <sup>2</sup>	88%	12%	4 / 1000 ft <sup>2</sup>	40%	60%	60.0 / 1000 ft <sup>2</sup>	50%	50%	Ashton Professional Centre
Retail	2.0 / 1000 ft <sup>2</sup>		60%	40%	6.0 / 1000 ft <sup>2</sup>	50%				Stadium Survey